GENERAL NOTES:

1. BUILDING AND DESIGN CODES:

A. 2018 INTERNATIONAL BUILDING CODE

B. AISC 360-106 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS (15TH EDITION STRUCTURAL STEEL MANUAL)

C. AISI S100-16 NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS,

D. AWS STRUCTURAL WELDING CODE-SHEET STEEL, D1.3-2008.

E. ACI 318 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, 2014

F. ACI DETAILING MANUAL, 2004

G. STRUCTURAL WELDED WIRE REINFORCEMENT MANUAL OF STANDARD PRACTICE, WIRE REINFORCEMENT INSTITUTE, 2011.

H. TMS 402/602 BUILDING CODE REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES, 2016

I. CRSI MANUAL OF STANDARD PRACTICE, 2009.

2. DESIGN LOADS:

A. LIVE LOADS: UNIFORM

ROOF	20 PSF
SLAB ON GRADE	100 PSF

LIVE LOAD REDUCTION ON SUPPORTING ELEMENTS IN ACCORDANCE WITH, BUILDING CODE

B. DEAD LOADS:

ROOF DEAD LOAD	20 PSF

C. WIND LOADS:

3 SECOND GUST WIND SPEED:	V = 115 MPH (ULT) 89 MPH (ASD)
EXPOSURE:	C
RISK CATEGORY:	II
ENCLOSURE CLASSIFICATION	ENCLOSED BUILDING
INTERNATIONAL PRESSURE COEFFICIENT	± 0.18

FOR COMPONENTS AND CLADDING WIND PRESSURE RE: 3/S-101

D. SEISMIC LOADS:

SEISMIC IMPORTANCE FACTOR: 1.0
OCCUPANCY CATEGORY: II
Ss: SPECTRAL ACC FOR SHORT PERIOD = 0.142g
Si: SPECTRAL ACC FOR 1 SECOND PERIOD = 0.043g
SITE CLASS D
SDS: DESIGN SPECTRAL ACC FOR SHORT PERIOD= 0.152g
SD1: DESIGN SPECTRAL ACC FOR 1 SECOND PERIOD= 0.069g
SEISMIC DESIGN CATEGORY B
Fa = 1.6
Fv = 2.4
BASIC SEISMIC FORCE RESISTING SYSTEM: LIGHT FRAMED WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE
DESIGN BASE SHEAR = 13.7 KIPS
Cs = 0.0234
R = 6.5
SEISMIC DESIGN PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE

E. SNOW LOADS
I: IMPORTANCE FACTOR = 1.0
Ce: EXPOSURE FACTOR = 1.0
Cr: THERMAL FACTOR = 1.0
Pg = GROUND SNOW LOAD = 40 PSF
Pf = 0.7 Ce Ct I Pg = 28 PSF
Pf = 28.0 PSF + SNOW DRIFT RE: 3/S-101- LOAD CASE 1
Pf = 28.0 PSF (MIN) - LOAD CASE 2
Pf = (RAIN ON SNOW SURCHARGE NOT APPLICABLE)) - LOAD CASE 3

F. ADDITIONAL DESIGN LOADS (ASD) INDICATED ON STRUCTURAL DRAWINGS SHALL BE IDENTIFIED AS FOLLOWS:

DL = DEAD LOAD
LL = LIVE LOAD
WL = WIND LOAD
EL = SEISMIC LOAD

3. GENERAL REQUIREMENTS:

A. SPECIFICATIONS ARE LISTED IN THE NOTES FOUND ON THIS SHEET. NO ADDITIONAL SPECIFICATION MANUAL IS INCLUDED FOR STRUCTURAL WORK.

B. VERIFY EXISTING CONDITIONS AND DIMENSIONS PRIOR TO BEGINNING WORK OR FABRICATING MATERIAL NOTIFY A/E OF DISCREPANCIES BEFORE PROCEEDING WITH ANY PHASE OF WORK.

C. VERIFY THE LOCATION OF CHASES, INSERTS, OPENINGS, SLEEVES, FINISHES, DEPRESSIONS, PADS, AND WALL OPENINGS.

D. DO NOT SCALE DRAWINGS FOR THE PURPOSE OF ESTABLISHING DIMENSIONS.

E. DETAILS LABELED "TYPICAL DETAILS" ON DRAWINGS APPLY TO SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY DETAILED . SUCH DETAILS APPLY WHETHER OR NOT DETAILS ARE REFERENCED AT EACH LOCATION . NOTIFY ENGINEER OF CONFLICTS REGARDING APPLICABILITY OF "TYPICAL DETAILS".

F. DO NOT LOAD THE SLAB ON GRADE OR SUPPORTED SLAB WITH ERECTION CRANES OR ERECTION EQUIPMENT. THE SLABS HAVE NOT BEEN DESIGNED FOR CRANE LOADS AND WILL REQUIRE AN INCREASE IN THICKNESS AND/OR REINFORCEMENT OBTAIN AND/ APPROVAL ON PROPOSED CRANE SUPPORT PLAN FOR SLABS PRIOR TO COMMENCING WORK.

G. DO NOT STORE OR STACK CONSTRUCTION MATERIALS ON POURED OR ERECTED FLOORS/ROOFS IN EXCESS OF 80 PERCENT OF LIVE LOAD. GENERAL CONTRACTOR WILL ENSURE THAT ALL SUB-CONTRACTORS ARE INFORMED OF LOADING RESTRICTIONS.AVOID IMPACT WHEN PLACING MATERIALS ON POURED OR ERECTED FLOORS OR ROOF.

H. THE CONTRACT STRUCTURAL DOCUMENTS REPRESENT THE FINISHED STRUCTURE. THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION. PROVIDE ALL MEASURES REQUIRED TO PROTECT THE STRUCTURE, WORKMEN, AND OTHER PERSONS DURING CONSTRUCTION; INCLUDING BRACING, SHORING FOR CONSTRUCTION EQUIPMENT, SHORING FOR THE BUILDING, FORMS AND SCAFFOLDING, SHORING OF RETAINING WALLS AND OTHER TEMPORARY SUPPORTS AS REQUIRED. COMPLY WITH APPLICABLE REQUIREMENTS OF OSHA AND OTHER GOVERNING BODIES HAVING JURISDICTION AT THE SITE.

I. PRINCIPAL OPENINGS THROUGH THE FRAMING ARE SHOWN ON DRAWINGS. EXAMINE THE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR THE REQUIRED OPENINGS AND PROVIDE FOR REQUIRED OPENINGS WHETHER SHOWN ON THE STRUCTURAL DRAWINGS OR NOT, VERIFY SIZE AND LOCATION OF OPENINGS WITH THE MECHANICAL CONTRACTOR. DEVIATIONS FROM THE STRUCTURAL DRAWINGS MUST BE APPROVED PRIOR TO IMPLEMENTING THE CHANGES.

J. LOADINGS FOR MECHANICAL EQUIPMENT ARE BASED ON THE UNITS SHOWN ON THE MECHANICAL DRAWINGS. ANY CHANGES IN TYPE, SIZE, OR NUMBER OF PIECES OF EQUIPMENT SHALL BE REPORTED TO THE ARCHITECT FOR VERIFICATION OF THE ADEQUACY OF SUPPORTING MEMBERS PRIOR TO THE PLACEMENT OF SUCH EQUIPMENT.

K. SEE ARCHITECTURAL DRAWINGS FOR ELEVATIONS NOT SHOWN AND FOR EXACT LOCATIONS OF ALL SLAB DEPRESSIONS. THE CONTRACTOR SHALL COMPARE THE STRUCTURAL SECTIONS WITH ARCHITECTURAL SECTIONS AND REPORT ANY DISCREPANCY TO THE ARCHITECT PRIOR T0 FABRICATING OR INSTALLING STRUCTURAL MEMBERS.

L. SEE ARCHITECTURAL DRAWINGS FOR THE FOLLOWING:

1) SIZE AND LOCATION OF ALL DOOR AND WINDOW OPENINGS, EXCEPT AS NOTED.
2) SIZE AND LOCATION OF ALL INTERIOR AND EXTERIOR NON-LOAD BEARING PARTITIONS.
3) SIZE AND LOCATION OF ALL CONCRETE CURBS, FLOOR DRAINS SLOPES, INSERTS, ETC. EXCEPT AS SHOWN.
4) FLOOR AND ROOF FINISHES.
5) WATERPROOFING AND DAMPPROOFING.
6) DIMENSIONS NOT SHOWN ON STRUCTURAL DRAWINGS.

M. SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR THE FOLLOWING:

1) PIPE AND DUCT RUNS, SLEEVES, HANGERS, TRENCHES, WALL AND SLAB OPENINGS ETC. EXCEPT AS SHOWN OR NOTED.
2) ELECTRICAL CONDUIT RUNS, BOXES, OUTLETS IN WALLS AND SLABS.
3) CONCRETE INSERTS FOR ELECTRICAL, MECHANICAL OR PLUMBING FIXTURES.
4) SIZE AND LOCATION OF MACHINE OR EQUIPMENT BASES, ANCHOR BOLTS MOTOR MOUNTS, EXCEPT AS SHOWN OR NOTED.

N. ALL MECHANICAL SYSTEMS SUSPENDED LOADS EXCEEDING 100 # SHALL BE REVIEWED BY THE STRUCTURAL ENGINEER PRIOR TO INSTALLATION UNLESS SPECIFICALLY DETAILED OR NOTED ON THE STRUCTURAL DRAWINGS.

FOUNDATION:

1.FOUNDATION DESIGN IS BASED UPON THE GEOTECHNICAL ENGINEERING REPORT ENTITLED "GEOTECHNICAL ENGINEERING REPORT PROJECT CT #1289 WESTMINSTER, MARYLAND PREPARED BY TERRACON CONSULTANTS. INC. DATED AUGUST 21, 2019.

COORDINATE STRUCTURAL PLANS AND DETAILS WITH REQUIREMENTS OF GEOTECHNICAL REPORT. FOOTING DESIGN IS BASED ON A 2000 PSF NET ALLOWABLE SOILS PRESSURE.

2. CONTRACTOR SHALL TREAT SOIL BELOW SLAB FOR TERMITES.

3. REFER TO THE GEOTECHNICAL REPORT AND SPECIFICATIONS FOR GENERAL REQUIREMENTS OF EARTHWORK, OVEREXCAVATION, SUBGRADE PREPARATION, FILL AND COMPACTION, WATERPROOFING AND OTHER PERTINENT REQUIREMENTS AND INFORMATION. IF THERE IS A CONFLICT BETWEEN GEOTECHNICAL REPORT AND STRUCTURAL PLANS THEN THE MORE STRINGENT CRITERIA SHALL APPLY UNLESS OTHERWISE DIRECTED BY AN RFI.

4. PROTECT PIPES AND CONDUITS RUNNING THROUGH WALLS AND SLABS WITH 1/2 INCH EXPANSION MATERIAL. LOWER CONTINUOUS FOOTINGS AND GRADE BEAMS PERPENDICULAR TO PIPE RUNS TO ALLOW PIPES TO PASS ABOVE THE FOOTINGS OR THROUGH THE GRADE BEAMS. ALTERNATIVELY, PROVIDE A CONCRETE JACKET IF PIPES ARE LOW ENOUGH TO BE PLACED BELOW THE FOOTINGS AND GRADE BEAMS. LOWER FOOTINGS AND GRADE BEAMS PARALLEL TO PIPE RUNS TO AVOID SURCHARGE ONTO ADJACENT TRENCH EXCAVATIONS.

5. MAINTAIN SUBGRADE AND FILL MOISTURE CONTENT UNTIL FOUNDATIONS ARE PLACED.

6. ARRANGE FOR OWNER'S INDEPENDENT TESTING AGENCY TO MONITOR CUT AND FILL OPERATIONS AND PERFORM FIELD DENSITY AND MOISTURE CONTENT TESTS TO VERIFY COMPACTION AND APPROVE FOOTING SUBGRADES PRIOR TO PLACING CONCRETE.

7. DO NOT PLACE FOOTINGS OR SLABS AGAINST SUBGRADE CONTAINING FREE WATER, FROST,OR ICE.

8. MAINTAIN PROPER SITE DRAINAGE DURING CONSTRUCTION TO ENSURE SURFACE RUNOFF AWAY FROM STRUCTURES AND TO PREVENT PONDING OF SURFACE RUNOFF NEAR THE STRUCTURES.

CONCRETE:

1. PROVIDE BATCH MIXING, TRANSPORTATION, PLACING AND CURING OF CONCRETE IN ACCORDANCE WITH RECOMMENDATIONS OF ACI 301 AND ACI 318. USE ASTM C 150 CEMENT UNLESS NOTED OTHERWISE. PROVIDE ADMIXTURES AND SPECIAL REQUIREMENTS AS SPECIFIED.

A. ALL CONCRETE SHALL BE NORMAL WEIGHT (145 PCF) CONCRETE AND: f'c=4,000 PSI AT 28 DAYS FOR ALL CONCRETE

2. MINIMUM CEMENT CONTENT SHALL BE 470 LBS (5 SACKS) PER CUBIC YARD.

3. ASTM C618 CLASS C OR F FLY ASH MAY BE USED UP TO 20% OF TOTAL CEMENT CONTENT BY VOLUME. THIS IS ONLY FOR CONCRETE SPECIFIED IN THESE STRUCTURAL DRAWINGS. REFER TO SPECIFICATIONS BY OTHER DISCIPLINES.

4. MAXIMUM SLUMP SHALL BE 5 IN., U.N.O.

5. CONCRETE SLUMP TESTS SHALL BE MADE BEFORE AND AFTER THE ADDITION OF ADMIXTURES AND MAY BE TAKEN AT THE BACK OF THE TRUCK. CONCRETE FOR THE PREPARATION OF TEST CYLINDERS SHALL BE TAKEN FROM THE HOSE END FOR CONCRETE PLACED BY PUMP.

6. CONCRETE MIX SHALL NOT USE ANY ADMIXTURE WHICH CONTAINS CALCIUM CLORIDE.

7. PREPARE AND SUBMIT MIXTURES FOR EACH CLASS OF CONCRETE ON THE BASIS OF LABORATORY TRIAL MIXTURES OR FIELD TEST DATA FOR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER

8. PROVIDE CONSTRUCTION AND CONTROL JOINTS AS INDICATED ON DRAWINGS. HORIZONTAL CONSTRUCTION JOINTS ARE NOT ALLOWED UNLESS SPECIFICALLY NOTED OR APPROVED BY STRUCTURAL ENGINEER. NOTIFY STRUCTURAL ENGINEER OF PROPOSED CONSTRUCTION JOINT OR CONTROL JOINT LOCATIONS THAT ARE DIFFERENT OR IN ADDITION TO JOINTS INDICATED ON DRAWINGS.

9. CHAMFER EXPOSED EDGES 3/4 INCH UNLESS OTHERWISE NOTED.

10. WIRE BRUSH AND CLEAN CONSTRUCTION JOINTS PRIOR TO POURING NEW CONCRETE.

11. REFERENCE THE APPROPRIATE DISCIPLINE DRAWINGS FOR SUBSLAB PIPING, FLOOR DRAINS AND SLAB AND WALL PENETRATIONS.

12. PROVIDE ADEQUATE STRUCTURAL FRAMING AS APPROVED BY STRUCTURAL ENGINEER FOR MECHANICAL OPENINGS THROUGH THE SLABS, WALLS AND FLOOR DECK. OPENINGS WILL NOT BE PERMITTED THROUGH BEAMS UNLESS SPECIFICALLY DETAILED.

13. ADMIXTURES:

A. AIR-ENTRAINING ADMIXTURE: ASTM C 260

B. WATER-REDUCING ADMIXTURE: ASTM C 494, TYPE A

C. RETARDING ADMIXTURE: ASTM C 494 TYPE B

D. ACCELERATING ADMIXTURE: ASTM C 494 TYPE C (NON CHLORIDE)

E. HIGH-RANGE, WATER REDUCING ADMIXTURE: ASTM C 494, TYPE F

H. OTHER ADMIXTURES FOR SPECIFIC USE WITH THE PERMISSION OF THE DESIGN PROFESSIONAL

F. WATER REDUCING AND ACCELERATING ADMIXTURE: ASTM C 494, TYPE E

G. WATER REDUCING AND RETARDING ADMIXTURE: ASTM C 494, TYPE D

14. COMPLY WITH RECOMMENDATIONS IN ACI 302.1R FOR SCREEDING, RESTRAIGHTENING AND FINISHING OPERATIONS FOR CONCRETE SURFACES. DO NOT WET CONCRETE.

A. INTERIOR GARAGE FLOOR SLABS: SMOOTH TROWEL FINISH

B. EXTERIOR SLABS: LIGHT FLEXIBLE BRISTLE BROOM FINISH

15. PROVIDE ACI "CLASS A" TOLERANCE, 1/8 INCH VARIATION IN 10 FEET. MEASURED WITH A STRAIGHT EDGE LAID IN ANY DIRECTION.

16. PROVIDE BATCH MIXING, TRANSPORTATION, PLACING AND CURING OF CONCRETE IN ACCORDANCE WITH RECOMMENDATIONS OF ACI 301 AND ACI 318. USE TYPE II OR TYPE V PORTLAND CEMENT UNLESS NOTED OTHERWISE. PROVIDE ADMIXTURES AND SPECIAL REQUIREMENTS AS SPECIFIED.

17. NO WATER SHALL BE ADDED TO THE CONCRETE AT THE JOBSITE. IT SHALL BE THE RESPOSIBILITY OF THE CONTRACTOR TO COORDINATE WITH THE CONCRETE SUPPLIER TO ENSURE A PUMPABLE AND WORKABLE MIX WITHOUT THE ADDITION OF WATER AT THE JOBSITE. THE USE OF PLASTICIZERS, RETARDANTS, AND OTHER ADDITIVES SHALL BE AT THE OPTION OF THE CONTRACTOR SUBJECT TO THE APPROVAL OF THE STRUCTURAL ENGINEER. FOLLOW THE RECOMMENDATIONS OF THE MANUFACTURER FOR THE PROPER USE OF ADDITIVES. THE USE OF CALCIUM CHLORIDE OR OTHER CHLORIDE BEARING SALTS SHALL NOT BE PERMITTED.

MASONRY:

1. REINFORCED MASONRY WORK AND MATERIALS TO BE IN ACCORDANCE WITH THE BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES: ACI 530/ASCE 5/TMS 402.

2. REINFORCED MASONRY TO CONFORM TO THE SPECIFICATIONS FOR MASONRY STRUCTURES: ACI 530.1/ASCE 6/TMS 602 (WITH THE EXCEPTIONS NOTED IN JOB SPECIFICATIONS).

3. PROVIDE CONCRETE MASONRY UNITS (CMU) OF NORMAL WEIGHT (125 PCF MINIMUM), GRADE N, TYPE I OR II, CONFORMING TO LATEST EDITION OF ASTM C 90. LAY UNITS IN RUNNING BOND UNLESS OTHERWISE NOTED.

4. PROVIDE MASONRY ASSEMBLAGES WITH MINIMUM PRISM STRENGTH (f'm) OF 1,500 PSI, TESTED IN ACCORDANCE WITH ASTM C 140.

5. PROVIDE CONCRETE MASONRY UNITS IN ACCORDANCE ASTM C 426 LIMITS FOR DRYING SHRINKAGE OF CONCRETE BLOCKS.

6. PROVIDE VERTICAL REINFORCEMENT IN CMU WALLS AS SHOWN IN DRAWINGS. FILL THE REINFORCED CELLS SOLID WITH GROUT. MAXIMUM HEIGHT OF GROUT POURS TO BE AS PER THE SPECIFICATION FOR MASONRY STRUCTURES TABLE NO. 7. UNTIL WALL IS PERMANENTLY BRACED BY ROOF.

7. LAY HOLLOW UNITS WITH FULL MORTAR COVERAGE ON HORIZONTAL AND VERTICAL FACE SHELLS. PROVIDE FULL MORTAR COVERAGE FOR WEBS WHEN ADJACENT TO GROUTED CELLS.

8. ALIGN VERTICAL CELLS TO BE FILLED WITH GROUT TO PROVIDE CONTINUOUS UNOBSTRUCTED VERTICAL CELLS. REMOVE OVERHANGING MORTAR OR OTHER OBSTRUCTION AND DEBRIS FROM THE INSIDES OF CELL WALLS. PROVIDE GROUT WITH 8 INCH SLUMP AND CONSOLIDATE BY MEANS OF HAND TAMPING TO ENSURE COMPLETE FILLING OF CELLS.

9. INSTALL ANCHORS, ACCESSORIES, AND OTHER ITEMS TO BE BUILT IN AS WORK PROGRESSES.

MASONRY CONT:

10. PERFORM CUTTING AND FITTING OF MASONRY WITH MASONRY SAWS PROVIDING CUT FINISHED UNITS.

11. CELLS AT OR BELOW FINISHED GRADE ARE TO BE GROUTED SOLID.

12. WHEN A FOUNDATION DOWEL DOES NOT LINE UP WITH A VERTICAL CORE, DO NOT SLOPE DOWEL MORE THAN ONE HORIZONTAL TO SIX VERTICAL.

13. WALL SHALL RECEIVE TEMPORARY BRACING. TEMPORARY BRACING SHALL NOT BE REMOVED UNTIL WALL IS PERMANENTLY BRACED BY ROOF.

14. SPECIAL INSPECTION IS REQUIRED AS FOLLOWS:

A. DURING PREPARATION OF REQUIRED PRISMS OR TEST SPECIMENS.
B. DURING THE LAYING OF MASONRY UNITS.
C. DURING PLACEMENT OF REINFORCING STEEL.
D. FOR GROUT SPACES PRIOR TO CLOSING OF CLEANOUTS AND GROUTING.
E. DURING ALL GROUTING OPERATIONS.

FORWARD INSPECTION RESULTS TO THE ENGINEER OF RECORD.

15. GROUT FILL CORES SHALL CONFORM TO ASTM C478 WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI IN 28 DAYS.

16. MORTAR SHALL COMFORM TO ASTM C270
A. MASONRY BELOW GRADE: TYPE M MORTAR
B. EXTERIOR ABOVE GRADE MASONRY: TYPE S MORTAR

17. GALVANIZED HORIZONTAL REINFORCEMENT SHALL HAVE 9 GAGE SIDE AND CROSS RODS SPACED 16" ON CENTER. LAP REINFORCEMENT 7".

REINFORCING STEEL:

1. PROVIDE DETAILING, FABRICATION, AND INSTALLATION OF REINFORCING AND ACCESSORIES IN ACCORDANCE WITH ACI 315 AND ACI 318.

2. PROVIDE NEW BILLET STEEL REINFORCING BARS IN ACCORDANCE WITH ASTM A 615, GRADE 60 DEFORMED.

3. COORDINATE PLACEMENT OF CAST-IN-PLACE EMBEDS AND ANCHOR RODS. SET ANCHOR RODS WITH A TEMPLATE. SECURELY ATTACH EMBED ITEMS TO FORMWORK OR REINFORCING.

4. PROVIDE CLASS "B" REINFORCEMENT SPLICES FOR CONTINUOUS REINFORCEMENT. PROVIDE STANDARD 90-DEGREE HOOKS IN ACCORDANCE WITH ACI 318, UNLESS OTHERWISE NOTED.

5. MAINTAIN THE FOLLOWING CONCRETE COVERAGE FOR REINFORCING STEEL UNLESS OTHERWISE NOTED:
A. CONCRETE CAST AGAINST EARTH: 3 INCHES
B. CONCRETE EXPOSED TO WEATHER
NO. 6 AND LARGER: 2 INCHES
NO. 5 AND SMALLER: 1 1/2 INCHES
C. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH THE GROUND:
SLABS AND WALLS
NO. 14 AND NO. 18: 1 1/2 INCHES
NO. 11 AND SMALLER: 3/4 INCHES

6. DO NOT WELD OR BEND REINFORCEMENT IN THE FIELD UNLESS SPECIFICALLY SHOWN OR APPROVED BY STRUCTURAL ENGINEER.

7. WHEN SPECIFICALLY APPROVED, PROVIDE WELDED REINFORCEMENT IN ACCORDANCE WITH ASTM A 706 GRADE 60. USE LOW HYDROGEN ELECTRODES FOR WELDING OF REINFORCEMENT IN CONFORMANCE WITH "RECOMMENDED PRACTICES FOR WELDING REINFORCING STEEL", AMERICAN WELDING SOCIETY, AWS D12.1. PROVIDE ASTM GRADE 40 REINFORCING BARS WHERE DETAILED BARS ARE TO BE WELDED TO A STEEL SECTION.

8. WHERE REQUIRED, PROVIDE DOWELS TO MATCH SIZE AND SPACING OF MAIN REINFORCING.

9. PROVIDE CONTINUOUS HORIZONTAL WALL REINFORCEMENT WITH 90-DEGREE BENDS AND EXTENSIONS AT CORNERS AND INTERSECTIONS AS SHOWN ON TYPICAL BAR PLACING DETAILS.

10. WHEN SHOWN ON DRAWINGS PROVIDE FIBRILLATED POLYPROPYLENE FIBERS ENGINEERED AND DESIGNED FOR USE IN CONCRETE COMPLYING WITH ASTM C1116, TYPE III, 1/2 TO 1 1/2 INCHES LONG.

POST INSTALLED ANCHORS

1. EXCEPT WHERE NOTED ON DRAWINGS, USE WWW.STRONGTIE.COM FOR ADDITIONAL PRODUCT DATA. IT IS ACCEPTABLE TO USE THE SIMPSON SET-XP ADHESIVE SYSTEM OR APPROVED EQUAL (TYP., U.N.O.) ICC ESR-2508.
2. EXCEPT WHERE INDICATED ON THE DRAWINGS, HILTI PRODUCTS MAY BE USED. CONTACT HILTI AT (800) 879-8000 FOR PRODUCT RELATED QUESTIONS.
3. ANCHOR CAPACITY USED IN DESIGN SHALL BE BASED ON THE TECHNICAL DATA PUBLISHED OR SUCH OTHER METHOD AS APPROVED BY THE STRUCTURAL ENGINEER OF RECORD. SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS MUST BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USE. CONTRACTOR SHALL PROVIDE CALCULATIONS DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERFORMANCE VALUES OF THE SPECIFIED PRODUCT. SUBSTITUTIONS WILL BE EVALUATED BY THEIR HAVING AN ICC ESR SHOWING COMPLIANCE WITH THE RELEVANT BUILDING CODE FOR SEISMIC USES, LOAD RESISTANCE, INSTALLATION CATEGORY, AND AVAILABILITY OF COMPREHENSIVE INSTALLATION INSTRUCTIONS. ADHESIVE ANCHOR EVALUATION WILL ALSO CONSIDER CREEP, IN-SERVICE TEMPERATURE AND INSTALLATION TEMPERATURE.
4. INSTALL ANCHORS PER THE MANUFACTURER INSTRUCTIONS, AS INCLUDED IN THE ANCHOR PACKAGING.
5. INSTALL ACCORDING TO MANUFACTURER'S SPECIFICATIONS. THREADED ROD AND REBAR DIAMETERS AND EMBEDMENT LENGTHS SHALL BE AS NOTED ON DRAWINGS.
6. OVERHEAD ADHESIVE ANCHORS MUST BE INSTALLED USING PRODUCTS WHICH HAVE SPECIFIC APPLICATIONS THAT ARE INTENDED FOR OVERHEAD USE.
7. THE CONTRACTOR SHALL ARRANGE AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS SPECIFIED. THE STRUCTURAL ENGINEER OF RECORD MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS.
8. ANCHOR CAPACITY IS DEPENDANT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS.
9. EXISTING REINFORCING BARS IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. UNLESS NOTED ON THE DRAWINGS THAT THE BARS CAN BE CUT, THE CONTRACTOR SHALL REVIEW THE EXISTING STRUCTURAL DRAWINGS AND SHALL UNDERTAKE TO LOCATE THE POSITION OF THE REINFORCING BARS AT THE LOCATIONS OF THE CONCRETE ANCHORS, BY FERROSCAN, GPR, X-RAY, CHIPPING OR OTHER MEANS.

LIGHT GAGE METAL FRAMING

1. STEEL STUD SIZES SHALL BE AS NOTED ON THE DRAWINGS, CONFORMING TO SSMA STANDARDS.
2. FABRICATE AND ERECT AS SPECIFIED. MANUFACTURER SHALL SUPPLY ALL CLIPS, FASTENERS, TEMPORARY BRACING, ACCESSORIES, STANDARD BRIDGING, ETC.
3. ALL STEEL STUDS SHALL HAVE A MINIMUM 1 5/8" FLANGE WITH A 1/2" RETURN (U.N.O.)
4. ALL STUDS 43 MIL (18 GAGE) AND THINNER SHALL HAVE A MINIMUM YIELD STRENGTH, Fy, OF 33 KSI AND TENSILE STRENGTH, Fu, OF 45 KSI. ALL STUDS 54 MIL (16 GAGE) AND THICKER SHALL HAVE A MINIMUM STRENGTH OF 50 KSI AND TENSILE STRENGTH OF 65 KSI (U.O.N.).
5. TRACK SECTIONS SHALL BE EQUAL GRADE AND GAGE THICKNESS OF STUDS BEING USED. TYPICA U.N.O.
6. ALL FASTENERS SHALL BE SELF-TAPPING NO. 12-14 GAGE SCREWS, OR WELD IN ACCORDANCE WITH SECTION 6.0 OF THE AMERICAN WELDING SOCIETY'S "STRUCTURAL WELDING CODE - SHEET STEEL " (AWS D1.3) AS SHOWN ON DRAWINGS.

STRUCTURAL WOOD

1. WOOD FRAMING SHALL COMPLY WITH THE SOUTHERN PINE INSPECTION BUREAU, OR SHALL CONFORM TO SPECIFICATIONS AS PUBLISHED BY THE WESTERN WOODS PRODUCTS ASSOCIATION.
2. WOOD FRAMING 2 INCHES X 4 INCHES AND LARGER SHALL BE NO. 1 DENSE SOUTHERN PINE, NO. 1 & BETTER DOUGLAS FIR LARCH, OR EQUIVALENT.
3. WOOD COLUMNS 6 INCHES X 6 INCHES AND LARGER SHALL BE NO. 1 DENSE SOUTHERN PINE, NO. 1 & BETTER DOUGLAS FIR LARCH, OR EQUIVALENT.
4. ALL EXPOSED WOOD RAFTERS AND COLUMNS SHALL BE "SELECT" GRADE AS DESCRIBED IN AITC.
5. ALL PLATES IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE TREATED (USE CATEGORY 2 AS SPECIFIED BY AWPA) FOR MOISTURE PROTECTION. ALL WOOD EXPOSED TO WEATHER SHALL BE PRESSURE TREATED (USE CATEGORY 3B AS SPECIFIED BY AWPA) OR WESTERN RED CEDAR.
6. GLUED-LAMINATED MEMBERS SHALL BE DOUGLAS FIR INDUSTRIAL GRADE (24F-V8) OR APPROVED EQUAL U.N.O. MEMBER SIZES ARE BASED ON CURRENT NDS WESTERN SPECIES. GRADE 24F-V4 MAY BE SUBSTITUTED FOR SIMPLY SUPPORTED SPANS ONLY.

STRUCTURAL WOOD CONT:

7. PRE-FABRICATED METAL PLATED WOOD TRUSSES
A. TRUSS FABRICATION AND INSTALLATION SHALL COMPLY WITH THE FOLLOWING STANDARDS:
1. ANSI/TPI 1 "NATIONAL DESIGN STANDARD FOR METAL-PLATE-CONNECTED WOOD TRUSS CONSTRUCTION".
2. TPI HIB "COMMENTARY AND RECOMMENDATIONS FOR HANDLING INSTALLING & BRACING METAL PLATE CONNECTED WOOD TRUSSES".
3. TPI DSB "RECOMMENDED DESIGN SPECIFICATION FOR TEMPORARY BRACING OF METAL PLATE CONNECTED WOOD TRUSSES".

B. TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING LOADS:
1. TOP CHORD: LIVE LOAD =28 PSF UNIFORM OR SNOW LOAD
DEAD LOAD =10 PSF

SNOW LOADS-PER LOAD CASES 1 TO 3 UNDER GENERAL NOTES, DESIGN LOADS SNOW LOADS NOTE 2E THIS SHEET

2. BOTTOM CHORD: LIVE LOAD =5 PSF (NON-CONCURENT)
DEAD LOAD =10 PSF
3. REFERENCE ROOF PLAN SHT S-102 FOR ADDITIONAL MECHANICAL EQUIPMENT LOADS.
4. REFERENCE 7/S-301 FOR LOADS FROM PARAPET BRACE.
5. REFERENCE ROOF PLAN SHT S-102 FOR ADDITIONAL LOADS FROM HANGING SOFFITS.
6. REQUIRED UPLIFT REFERENCE LOAD DIAGRAM DETAILS - DETAIL 4 ON S-102

C. ALL TRUSS-TO-TRUSS AND TRUSS FRAMING CONNECTORS SHALL BE SPECIFIED AND DESIGNED BY THE TRUSS MANUFACTURER.
D. BOTTOM CHORD TENSION SPLICES ARE PROHIBITTED WITHIN THE MIDDLE 1/3 OF THE SPAN.
E. FABRICATE, SUPPLY AND ERECT WOOD TRUSSES AS SHOWN ON THE DRAWINGS AND AS SPECIFIED. WORK TO INCLUDE ANCHORAGE, BLOCKING, CURBING, MISCELLANEOUS INCIDENTAL FRAMING AND CONSTRUCTION AND PERMANENT BRACING
F. FABRICATOR SHALL DESIGN AND FURNISH ALL TRUSS TO TRUSS HANGERS AND CONNECTIONS HARDWARE INCLUDING INSTALLATION INFORMATION
G. ALL LOADS SHALL BE LOCATED AT TRUSS TOP OR BOTTOM CHORD PANEL POINTS
H. COORDINATE SIZE AND LOCATIONS FOR MECHANICAL AND ELECTRICAL DUCTWORK PRIOR TO CONSTRUCTION
I. MAXIMUM ALLOWABLE LIVE LOAD DEFLECTION (UNLESS OTHERWISE NOTED ON PLAN) ROOF TRUSSES - SPAN/360
J. OPENINGS IN ROOF TO BE DESIGNED/FRAMED BY THE TRUSS SUPPLIER
K. VERIFY BEARING CONDITIONS PRIOR TO TO TRUSS FABRICATION
L. PROVIDE BRIDGING, WEB STIFFENERS, ETC. AS REQUIRED
M. FABRICATOR SHALL SUBMIT SHOP DRAWINGS SHOWING LAYOUT OF MEMBERS, BRIDGING, BRACING, ERECTION DETAILS, TRUSS PENETRATIONS, AND DESIGN CALCULATIONS SEALED BY A REGISTERED ENGINEER IN THE STATE OF MARYLAND. SUBMITTALS WHICH DO NOT INCLUDE THE ABOVE INFORMATION WILL BE RETURNED TO THE CONTRACTOR PRIOR TO REVIEW

8. ROOF DECK
A. ALL ROOF DECK SHALL BE APA STRUCTURAL 1 GRADE EXTERIOR PLYWOOD OR ORIENTED STRAND BOARD. COMFORMING TO GROUP 1 OF TABLE 1 OF APA PANEL DESIGN SPECIFICATION
B. ROOF SHEATHING SHALL BE 23/32 INCH THICK MINIMUM (48/24) U.N.O.
C. STAGGER ENDS OF SHEETS.
D. PROVIDE BLOCKING AT EDGES OF ALL ROOF SHEETS.
E. NAIL EDGES OF ROOF SHEETS AT 4 IN. O.C. MAXIMUM (U.N.O.).
F. NAIL FACES OF ROOF SHEETS AT 12 IN. O.C. MAXIMUM.
G. USE MINIMUM 10d COMMON NAILS (U.N.O.).

9. WALL SHEATHING
A. ALL WALL SHEATHING SHALL BE APA STRUCTURAL 1 GRADE EXTERIOR PLYWOOD OR ORIENTED STRAND BOARD. COMFORMING TO GROUP 1 OF TABLE 1 OF APA PANEL DESIGN SPECIFICATION
B. WALL SHEATHING SHALL BE 15/32 INCH THICK MINIMUM (32/16) U.N.O.
C. STAGGER ENDS OF SHEETS.
D. PROVIDE BLOCKING AT EDGES OF ALL SHEARWALL SHEETS.
E. NAIL EDGES OF SHEARWALL SHEETS PER SCHEDULE ON PLAN (OTHER WALLS AT 6 IN. O.C. MAXIMUM).
F. NAIL FACES OF WALL SHEETS AT 12 IN. O.C. MAXIMUM.
G. USE MINIMUM 10d COMMON NAILS (U.N.O.).

10. CONNECTORS SHALL BE AS MANUFACTURED BY THE SIMPSON CO. OR APPROVED EQUAL. CONNECTORS USED WITH PRESSURE TREATED LUMBER OR IN UNCONDITIONED SPACE, SHALL HAVE THE ZMAX (6185) COATING.

11. NAILING, UNLESS NOTED OTHERWISE, SHALL BE PER THE 2018 INTERNATIONAL BUILDING CODE. TABLE 2304.10.1

12. CONNECTORS SHALL BE AS MANUFACTURED BY THE SIMPSON CO. OR APPROVED EQUAL. CONNECTORS USED WITH PRESSURE TREATED LUMBER OR IN UNCONDITIONED SPACE, SHALL HAVE THE ZMAX (6185) COATING. ALL NAILS USED FOR CONNECTORS SHALL MATCH THOSE SPECIFIED BY THE SUPPLIER'S PRODUCT CATALOG.

NAILING SCHEDULE (PER IBC TABLE 2304.10.1) UON

CONNECTION	FASTENING (1)	LOCATION
JOIST TO SILL OR GIRDER	3-8d COMMON 3-3"x 0.131" NAILS	TOE NAIL
BRIDGING TO JOIST	2-8d COMMON 3-3"x 0.131" NAILS	TOE NAIL AT EA END
SILL PLATE TO JOIST OR BLOCKING	16d BOX @ 16" O.C. 3"x 0.131" @ 8" O.C.	FACENAIL
TOP PLATE TO STUD, END NAIL	2-16d COMMON 3-3"x 0.131" NAILS	END NAIL
STUD TO SILL PLATE	4-8d COMMON 4-3"x 0.131" NAILS	TOE NAIL
	2-16d COMMON 3-3"x 0.131" NAILS	END NAIL
DOUBLE STUD	16d BOX @ 16" O.C. 3"x 0.131" @ 8" O.C.	FACENAIL
DOUBLE TOP PLATES	16d BOX @ 24" O.C. 3"x 0.131" @ 12" O.C.	TYP FACENAIL
	8-16d COMMON 12-3"x 0.131" NAILS	LAP SPLICE
BLK'S BTWN JOISTS OR RAFTERS TO TOP PLATE	3-8d COMMON 3-3"x 0.131" NAILS	TOE NAIL
RIM JOIST TO TOP PLATES	8d COMMON @ 6" OC 3"x 0.131" NAILS @ 6" OC	TOE NAIL
TOP PLATES LAPS AND INTERSECTIONS	2-16d COMMON 3-3"x 0.131" NAILS	FACENAIL
CONTINUOUS HEADER, TWO PIECES	16d COMMON @ 16" OC	ALONG EA EDGE
CEILING JOISTS TO PLATE	3-8d COMMON 5-3"x 0.131" NAILS	TOE NAIL
CONTINUOUS HEADER TO STUD	4-8d COMMON	TOE NAIL
CEILING JOISTS, LAPS OVER PARTITIONS	3-16d COMMON 4-3"x 0.131" NAILS	FACENAIL
CEILING JOISTS PARALLEL RAFTERS	3-16d COMMON 4-3"x 0.131" NAILS	FACENAIL
RAFTER TO PLATE	3-8d COMMON 3-3"x 0.131" NAILS	TOE NAIL
BUILT-UP CORNER STUDS	16d COMMON @ 24" OC 3"x 0.131" NAILS @ 16" OC	
BUILT-UP GIRDER BEAMS	20d COMMON @ 32" OC 3-3"x 0.131" NAILS @ 24" OC	TOE NAIL T&B & STAGG'D
	2-20d COMMON 3-3"x 0.131" NAILS	FACENAIL AT ENDS & AT SPLICE

1.) NAIL SIZES ARE AS FOLLOWS:
8d COMMON = 2 1/2" x 0.131"
10d COMMON = 3" x 0.148"
16d COMMON = 3 1/2" x 0.162"
20d COMMON = 4" x 0.192"
16d BOX = 3 1/2" x 0.135"

ALL SILL PLATE NAILS OF SHEAR WALLS AND PLYWOOD NAILS SHALL BE COMMON NAILS UON

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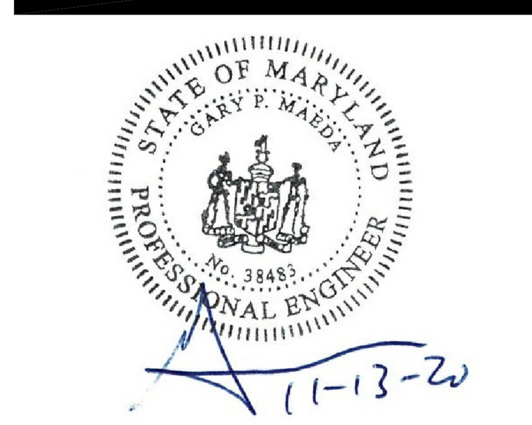
REVISIONS:

ISSUE DATE:

	PERMIT SET	2020-11-13

DRAWN BY: DWG

CFT PROJECT #:
ARCH PROJECT #: JCDT190143



I certify that these documents were prepared or approved by me, and that I am a duly licensed architect or engineer under the laws of the State of Maryland, license number 38483 , expiration date 02-17-2022 .

NORR
ARCHITECTS ENGINEERS PLANNERS

CFT PLAZA
S-19-0029

210 MALCOLM DR.
WESTMINSTER, MD 21157

S-000

STRUCTURAL NOTES AND SPECIAL INSPECTIONS

Statement of Special Inspections

This Statement of Special Inspections / Quality Assurance Plan includes the following building systems:

- ☑ Soils and Foundations
- ☑ Cast-In-Place Concrete
- ☐ Special Cases
- ☑ Structural Steel
- ☑ Wood Construction
- ☑ Masonry

General Notes

The Inspectors and testing agencies shall be engaged by the Owner or the Owner's Agent, and not by the Contractor or Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the Building Official, prior to commencing work.

The qualifications of all personnel performing Special Inspections and testing activities are subject to the approval of the Building Official and E.O.R. The credentials of all inspectors and testing technicians shall be provided if requested.

The special Inspectors shall keep records of Inspections and shall furnish Inspection reports to the owner, Engineer of Record (E.O.R.) and Architect of Record (A.O.R.). Field and testing result reports shall be submitted to all designated parties as they are completed. The reports shall indicate that the work performed was done in accordance to the construction drawings. Discrepancies shall be brought to the attention of the general contractor for correction. If the discrepancies are not corrected, the discrepancies shall be brought to the attention of the E.O.R. prior to completion of that phase of work. A final report that documents required special inspections and corrections of discrepancies shall be submitted by the General Contractor to the Owner, E.O.R. and A.O.R.

Soils and Foundations

Item	Scope	Monitoring: Periodic (P) Continuous (C)
1. Shallow Foundations	*Inspect soils below footings for adequate bearing capacity and consistency with geotechnical report.* *Inspect removal of unsuitable material and preparation of subgrade prior to placement of controlled fill.*	*P* *C*
2. Controlled Structural Fill	*Perform sieve tests (ASTM D422 & D1140) and modified Proctor tests (ASTM D1557) of each source of fill material.* *Inspect placement, lift thickness and compaction of controlled fill.* *Test density of each lift of fill by nuclear methods (ASTM D2922)* *Verify extent and slope of fill placement.*	*C*

Note:
1. Special Inspection is not required during placement of controlled fill having a total depth of 12 inches or less.

Cast-in-Place Concrete

Item	Scope	Monitoring: Periodic (P) Continuous (C)
1. Mix Design	*Review concrete batch tickets and verify compliance with approved mix design. Verify that water added at the site does not exceed that allowed by the mix design. Submit proposed mix design of each class of concrete to Structural Engineer of Record and to inspection and testing firm for review prior to commencement of work.*	*P*
2. Material Certification	*Review for conformance to contract documents. Submit to Structural Engineer of Record for review.*	*P*
3. Anchor Rods	*Inspect size, positioning and embedment of anchor rods. Inspect concrete placement and consolidation around anchors.*	*C*
4. Concrete Placement	*Inspect placement of concrete. Verify that concrete conveyance and depositing avoids segregation or contamination. Verify that concrete is properly consolidated.*	*C*
5. Sampling and Testing of Concrete	*Test concrete compressive strength (ASTM C31 & C39), slump (ASTM C143), air-content (ASTM C231 or C173) and temperature (ASTM C1064). Three concrete test cylinders will be taken for every 75 or less cubic yards of each class of concrete placed, or concrete placed on any given day. One additional test cylinder will be taken during cold weather concreting, cured on job site under same conditions as concrete represents.*	*C*
6. Curing and Protection	*Inspect curing, cold weather protection and hot weather protection procedures.*	*P*

Note: Special Inspection is not required for flatwork patios, driveways and sidewalks, on grade not shown on structural drawings.

Structural Steel

Item	Scope	Monitoring: Periodic (P) Continuous (C)
1. Material Certification	*Review for conformance to contract documents. Submit to Structural Engineer of Record for review.*	*P*
2. Anchor Rods and embedded plates	*Inspect size, positioning and embedment of anchor rods and embedded plates*	*C*
3. Welder Qualification	*Qaulification per AWS D1.1, Subclause 6.1.4., D1.1M*	*P*
4. Weld Acceptance	*Welds meet visual acceptance per AWS D1.1/D1.1M /C*	*C*

Wood Construction

Item	Scope	Monitoring: Periodic (P) Continuous (C)
1. Fabricator Certification/ Quality Control Procedures ☐ Fabricator Exempt	*Inspect shop fabrication and quality control procedures for wood truss plant.* *Confirm certification of supplier.*	*P*
2. Material Grading	*Inspect grade stamps on structural lumber for compliance with the contract documents.*	*P*
3. Connections	*Verify connection hardware and its installation. Inspect bearing, nails, bolts, hangers or clips, or other devices are tight and otherwise properly installed per the contract documents.*	*C*
4. Framing and Details	*Inspect members for size and placement for conformance to the SER approval submittals and contract document. Review engineered joist shop drawings. Submit to SER for review.*	*P*
5. Diaphragms & Shearwalls	*Inspect thickness and grade of plywood (or OSB), blocking, placement, embedment, size of hold down anchors and the edge and field nailing of the plywood (or OSB) to the framing for conformance to the contract documents.*	*C*
6. Prefabricated Wood Trusses	*Inspect the fabrication of wood trusses. Bottom chord splices are prohibited in the middle third point of the truss.*	*P*
7. Permanent Truss Bracing	*Bridging and bracing installed per the approved truss shop drawings.*	*P*

Masonry

Item	Scope	Monitoring: Periodic (P) Continuous (C)
1. Verify Compliance with the approved submittal	*Review submittals.*	*P*
2. Verification of F'M	*Review submittals.*	*P*
3. As masonry construction begins verify for comprlinace	*Verify.* *a. Portions of size prrepared mortar.* *b. Construction of mortar joints.* *c. location of connection.*	*P*
4. During construction	*Verify.* *a. Type, size and location of anchors, including other details of anchorage of masonry to structural members, frames or other construction.* *b. preparation construction and portion of masonry during cold weather (temperature below 40° F) or hot weather (temperature above 90°)*	*P*
5. Observe preparation of mortar specimens and / or prisms	*Field review.* *a. Preparation of mortar cubes* *b. Preparation of test prisms*	*P*

Special Cases

Item	Scope	Monitoring: Periodic (P) Continuous (C)
Epoxy Anchors In Concrete or CMU	*Review anchors and product being used for conformance to contract documents. Observe installation for compliance to manufacturers specifications. Perform pull test to 125% of allowable design load per manufacturer specifications. (Minimum of 10% of total anchors, to include a minimum of one of each type, size or embedment.)*	*C*

STRUCTURAL STEEL:

1. DESIGN, DETAIL AND ERECT STRUCTURAL STEEL ELEMENTS IN ACCORDANCE WITH THE FOLLOWING:
   A. AISC SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS.
   B. AISC MANUAL OF STEEL CONSTRUCTION, ALLOWABLE STRESS DESIGN.
   C. AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES.
   D. AWS STRUCTURAL WELDING CODE, D1.1.
2. PROVIDE STRUCTURAL STEEL OF THE FOLLOWING ASTM DESIGNATIONS UNLESS OTHERWISE NOTED:
   A. ANGLES, BENT PLATES, HANGERS AND BRACES: ASTM A 36
   B. STRUCTURAL PIPE: ASTM A 53, GRADE B, TYPE E OR S
   C. HOLLOW STRUCTURAL SHAPES: ASTM A 500, GRADE C
   D. BASE PLATES AND MISCELLANEOUS STEEL PLATES: ASTM A 36
   E. ANCHOR RODS: ASTM F 1554, GRADE 36
   F. W SECTONS ASTM A992
3. CONNECTION MATERIALS:
   A. BEAM-COLUMN STIFFENER PLATES AND DOUBLER PLATES S SHAPES TO MATCH THE GRADE STEEL OF STRUCTURAL ELEMENT
4. WELD MINIMUM SIZE AND STRENGTH:
   A. PROVIDE MINIMUM SIZE OF FILLET WELDS AS SPECIFIED IN TABLE J2.4 OF THE AISC MANUAL.
   B. PROVIDE MINIMUM EFFECTIVE THROAT THICKNESS OF PARTIAL PENETRATION GROOVE WELDS AS SPECIFIED IN TABLE J2.3 OF THE AISC MANUAL.
   C. PROVIDE ELECTRODES FOR FIELD OR SHOP WELDING THAT CONFORM TO ASTM A 233 (CLASS 70).
   D. ALL WELDS ARE CONTINUOUS FOR THE FULL LENGTH OF THE CONNECTION UNLESS OTHERWISE NOTED ON DRAWINGS.
5. PROVIDE MINIMUM OF TWO BOLTS PER CONNECTION. PROVIDE MINIMUM BOLT DIAMETER OF 3/4 INCH.
6. PROVIDE SIMPLE SHEAR CONNECTIONS FOR STEEL CONNECTIONS NOT SPECIFIED OTHERWISE UTILIZING HIGH STRENGTH BEARING BOLTS IN SINGLE SHEAR. PROVIDE SINGLE PLATE SHEAR TAB BOLTED CONNECTIONS.
7. STEEL FABRICATION:
   A. FABRICATE AND ASSEMBLE STRUCTURAL MEMBERS/ASSEMBLIES IN SHOP TO GREATEST EXTENT POSSIBLE.
   B. SPLICING OF STRUCTURAL STEEL MEMBERS IS PROHIBITED WITHOUT PRIOR APPROVAL BY THE A/E.
   C. BE RESPONSIBLE FOR ALL ERRORS OF DETAILING ON THE SHOP DRAWINGS, ERRORS IN FABRICATION, AND THE CORRECT FITTING OF STRUCTURAL STEEL MEMBERS.
   D. CONFORM TO THE AISC CODE OF STANDARD PRACTICE, FOR ERECTION TOLERANCES. FIELD MODIFICATION TO STRUCTURAL STEEL IS PROHIBITED WITHOUT PRIOR APPROVAL BY THE A/E.
   E. CLEAN STEEL OF RUST, LOOSE MILL SCALE AND OTHER FOREIGN MATERIALS WHERE REQUIRED FOR FABRICATION, FITTING UP, OR WELDING.
   F. DO NOT CUT STRUCTURAL STEEL MEMBERS FOR THE WORK OF OTHER TRADES WITHOUT PRIOR REVIEW AND APPROVAL OF THE A/E.
11. HOT DIP GALVANIZE AFTER FABRICATION ALL STRUCTURAL STEEL AND THEIR CONNECTIONS PERMANENTLY EXPOSED TO THE OUTSIDE. ITEMS INCLUDED BUT NOT LIMITED TO:
   A. SHELF ANGLES, LOOSE LINTELS
   B. EXAMINE THE ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR OTHER ITEMS THAT REQUIRE HOT DIPPED GALVANIZATION.
12. PROVIDE GROUT FOR BASE PLATES THAT IS NON-SHRINK, NON-METALLIC GROUT WITH MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 6000 PSI. COMPLETE GROUT WORK PRIOR TO PLACING ROOF CONCRETE OF A SINGLE STORY BUILDING OR PRIOR TO PLACING SECOND FLOOR CONCRETE OF A MULTIPLE STORY BUILDING.
13. PROVIDE WASHERS FOR ALL CONNECTIONS WITH STANDARD, OVERSIZE AND SHORT-SLOTTED HOLES. FOR LONG-SLOTTED HOLES PROVIDE WASHERS OR A CONTINUOUS BAR OF SUFFICIENT SIZE TO COMPLETELY COVER THE SLOT. PLATE WASHERS OR BARS TO BE MINIMUM OF 5/16 INCH THICK FOR LONG-SLOTTED HOLES.
14. WIDE FLANGE BEAM CONNECTIONS TO TUBE COLUMNS SHALL BE MADE WITH BOLTED SHEAR TAB PLATE TYPE CONNECTIONS UNLESS OTHERWISE NOTED ON PLAN. ONE-SIDED CONNECTIONS SHALL BE DESIGNED AS ECCENTRIC CONNECTIONS.
15. MILL STEEL COLUMN ENDS TO FIT FLUSH WITH BASE PLATE, CAP PLATE AND END PLATES. FIELD ASSEMBLY OF THESE STEEL ELEMENTS TO THE COLUMNS IS PROHIBITED.
16. HEADED STUDS (SHEAR AND ANCHOR) AND DEFORMED ANCHORS:
   A. PROVIDE HEADED STUDS (ANCHOR) MADE OF MATERIAL CONFORMING TO ASTM A 108.
   B. WELD STUDS ACCORDING TO MANUFACTURER'S RECOMMENDATIONS. MANUAL ARC (STICK) WELDING OF HEADED STUDS AND/OR DEFORMED ANCHORS IS NOT ALLOWED.
18. PROVIDE TEMPORARY BRACING DURING CONSTRUCTION PHASE, PRIOR TO COMPLETING CONNECTIONS. TEMPORARY CONSTRUCTION BRACING OF THE STRUCTURAL STEEL FRAME IS THE RESPONSIBILITY OF THE CONTRACTOR
19. CLEAN STEEL TO BE PAINTED IN ACCORDANCE WITH STEEL STRUCTURES PAINTING COUNCIL POWER TOOL CLEANED SSPC-SP3.

SPECIAL INSPECTION:

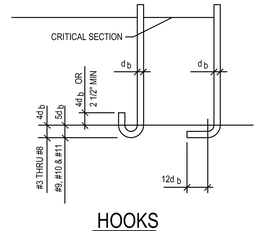
1. SPECIAL INSPECTION SHALL MEET THE REQUIREMENTS OF IBC SECTION 1704 AND TABLE LISTED ON THIS SHEET. SPECIAL INSPECTOR(S) SHALL BE HIRED BY THE OWNER TO PERFORM THE REQUIRED SPECIAL INSPECTIONS. THE NAMES OF PERSONS OR FIRMS WHO ARE TO PERFORM THE SPECIAL INSPECTIONS SHALL BE FORWARDED TO THE BUILDING OFFICIAL FOR APPROVAL. THE SPECIAL INSPECTOR(S) SHALL COMPLETE AND SUBMIT ALL FORMS REQUIRED BY THE CITY OF WESTMINSTER, MARYLAND.
2. ITEMS LISTED IN THE SPECIAL INSPECTION SCHEDULE SHALL BE INSPECTED IN ACCORDANCE WITH MEC CHAPTER 17 BY A CERTIFIED SPECIAL INSPECTOR FROM AN ESTABLISHED TESTING AGENCY.
3. FOR MATERIAL SAMPLING AND TESTING REQUIREMENTS, REFER TO THE MATERIAL SAMPLING AND TESTING SECTION, THE PROJECT SPECIFICATIONS, AND THE SPECIFIC GENERAL NOTES SECTIONS.
4. THE TESTING AGENCY SHALL SEND COPIES OF ALL STRUCTURAL TESTING AND INSPECTION REPORTS DIRECTLY TO THE REGISTERED DESIGN PROFESSIONAL, CONTRACTOR, AND BUILDING OFFICAL. ANY ITEMS WHICH FAIL TO COMPLY WITH THE APPROVED CONSTRUCTION DOCUMENTS SHALL IMMEDIATELY BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION.
5. IF DISCREPANCIES ARE NOT CORRECTED, THEY SHALL BE BROUGHT TO THE ATTENTION OF THE BULIDING OFFICIAL AND REGISTERED DESIGN PROFESSIONAL PRIOR TO COMPLETION OF THAT PHASE OF THE WORK. SPECIAL INSPECTION TESTING REQUIREMENTS APPLY EQUALLY TO ALL CONTRACTOR DESIGNED COMPONENTS
6. CONTINUOUS SPECIAL INSPECTION MEANS THE FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED.
7. PERIODIC SPECIAL INSPECTION MEANS THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECAIL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK (REFERENCE IBC SECTION 1702)
8. ALL WELDS SHALL BE VISUALLY INSPECTED IN ACCORDANCE WITH IBC SECTION 1705.2
9. ALL COMPLETE PENETRATION WELDS SHALL BE TESTED ULTRASONICALLY OR BY USING ANOTHER APPROVED METHOD (IBC 1705.2.1) IN ACCORDANCE WITH IBC SECTION 1705.2
10. WELDING INSPECTIONS SHALL BE IN COMPLIANCE WITH AWS D1.1
11. INSPECTION OF PRE-FABRICATED CONSTRUCTION SHALL BE THE SAME AS IF THE THE MATERIAL USED IN THE CONSTRUTION TOOK PLACE ON SITE. CONTINUOUS INSPECTION WILL NOT BE REQUIRED DURING PRE-FABRICATION IF THE APPROVED TESTING AGENCY CERTIFIES THE CONSTRUCTION AND FURNISHES EVIDENCE OF COMPLIANCE
12. ANY CONSTRUCTION OR MATERIAL THAT HAVE FAILED INSPECTION SHALL BE SUBJECT TO REMOVAL AND REPLACEMENT REQUIRED DURING PRE-FABRICATION IF THE APPROVED TESTING AGENCY CERTIFIES THE CONSTRUCTION AND FURNISHES EVIDENCE OF COMPLIANCE
13. SPECIAL INSPECTION OF SOILS SHALL REFERENCE THE APPROVED SOILS REPORT TO DETERMINE COMPLIANCE
14. SLIP-CRITICAL CONNECTIONS MAY HAVE PERIODIC SPECIAL INSPECTION PROVIDED THAT THE TURN-OF-THE-NUT METHOD WITH MATCHMARKING TECHNIQUES ARE USED

DEFERRED SUBMITTAL:

1. THE FOLLOWING ITEMS ARE DEFERRED SUBMITTALS, PENDING APPROVAL OF THE BUILDING OFFICIAL:
   A. RACKS INCLUDING LATERAL BRACING AND ANCHORAGE
   B. CORRESPONDING JOIST PLANS AND PIECE SUBMITTALS.
   C. DESIGN DRAWING FOR ATTACHMENT OF MECHANICAL AND ELECTRICAL COMPONENTS TO THE STRUCTURE AND SUPPORTED BY THE STRUCTURE.
   D. CONCRETE DESIGN MIXES.
   E. COLD FORMED METAL FRAMING SHOP DRAWINGS. CALCULATIONS MUST MAKE REFERENCE TO THE PLANS AND PIECE SUBMITTALS.
   F. MASONRY REINFORCING SHOP DRAWINGS.
   G. STRUCTURAL STEEL SHOP DRAWINGS. CALCULATIONS MUST MAKE REFERENCE TO THE PLANS AND PIECE SUBMITTALS.
   H. CONCRETE REINFORCING SHOP DRAWINGS.
   I. PRE-FABRICATED METAL PLATED WOOD TRUSS SHOP DRAWINGS.
   J. PANELIZED WOOD FRAMING SHOP DRAWINGS CALCULATIONS MUST MAKE REF TO THE THE PLANS AND PIECE SUBMITTALS
2. DEFERRED SUBMITTALS MUST BE ACCOMPANIED BY CALCULATIONS SIGNED BY A REGISTERED MARYLAND ENGINEER.
3. DEFERRED SUBMITTALS MUST BE SUBMITTED PRIOR TO INSTALLATION AND MUST BE APPROVED BY THE ENGINEER OF RECORD FOR GENERAL CONFORMANCE TO DESIGN ASSUMPTION INTENT.

CONCRETE REINFORCING SPLICE DEVELOPMENT LENGTH

	SPLICE LENGTHS (CLASS "B")				DEVELOPMENT LENGTH			
	ALL BARS EXCEPT TOP BARS		TOP BARS (HORIZ BARS WITH MORE THAN 12" OF CONC BELOW)		ALL BARS EXCEPT TOP BARS		TOP BARS (HORIZ BARS WITH MORE THAN 12" OF CONC BELOW)	
f'c (PSI) / BAR SIZE	INCHES		INCHES		INCHES		INCHES	
	3500	4000	3500	4000	3500	4000	3500	4000
#3	16	16	21	18	13	12	16	14
#4	22	19	28	24	17	15	22	19
#5	27	23	35	30	21	18	27	23
#6	43	37	56	48	33	29	43	37
#7	63	54	81	71	48	42	63	54
#8	72	62	93	81	55	48	72	62
#9	81	70	105	91	62	54	81	70
#10	89	77	116	101	69	60	89	77
#11	98	85	128	111	76	66	98	85



HOOKS

NOTES:

1. DEVELOPMENT AND SPLICE LENGTHS SHOWN ARE BASED ON THE TABLE PROVIDED IN ACI 318-14 SECTION 25.4.2.2 AND APPLY UNLESS OTHERWISE NOTED ON PLANS AND DETAILS.
2. ALL LENGTHS AND COVERS ARE GIVEN IN INCHES UNLESS OTHERWISE NOTED.
3. TABULAR VALUES FOR DEVELOPMENT AND SPLICE LENGTHS ARE BASED ON NORMAL WEIGHT CONCRETE, GRADE 60 UNCOATED REINFORCING, CLEAR SPACING OF BARS EQUAL TO OR GREATER THAN 2 x BAR DIA OR 1 1/2", WHICHEVER IS GREATER, AND CLEAR COVER EQUAL TO OR GREATER THAN 1 x BAR DIAMETER OR 3/4", WHICHEVER IS GREATER.
4. TOP BARS ARE HORIZONTAL BARS PLACED SUCH THAT MORE THAN 12" OF CONCRETE IS CAST IN THE MEMBER BELOW THE BAR. HORIZONTAL BARS IN WALLS ARE CONSIDERED AS TOP BARS. VERTICAL BARS MAY BE CONSIDERED AS OTHER BARS.
5. SPLICES SHALL NOT BE MADE AT POINTS OF MAXIMUM STRESS AS DETERMINED BY THE ENGINEER.

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REVISIONS:

ISSUE DATE:

PERMIT SET	2020-11-13

DRAWN BY: DWG

CFT PROJECT #:

ARCH PROJECT #: JCDT190143

STATE OF MARYLAND · GARY F. MARSH · No. 38483 · PROFESSIONAL ENGINEER

I certify that these documents were prepared or approved by me, and that I am a duly licensed architect or engineer under the laws of the State of Maryland, license number 38483 expiration date 02-17-2022

NORR ARCHITECTS | ENGINEERS | PLANNERS

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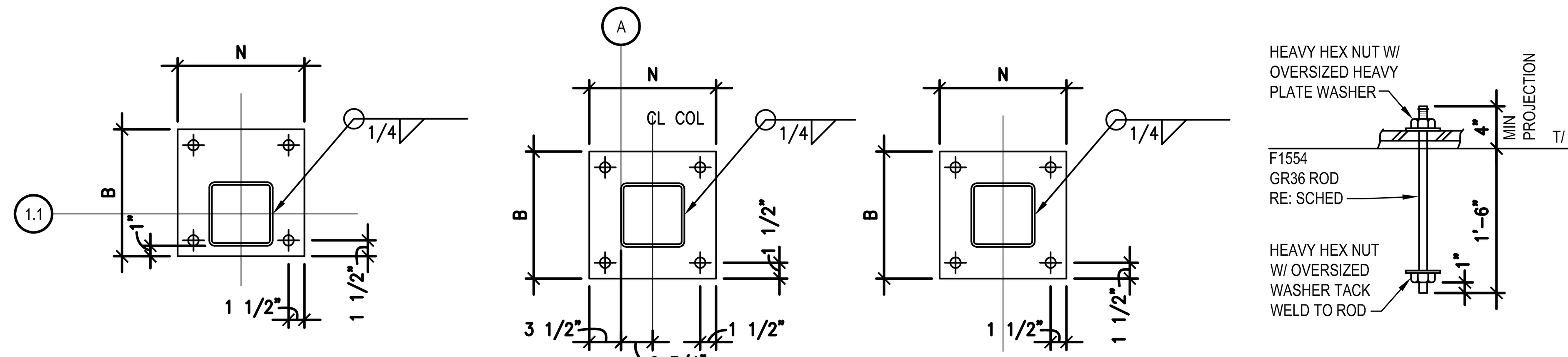
NOTES AND SPECIAL INSPECTIONS



TYP BASE PLATE PLAN C1 — C2A BASE PLATE PLAN — TYP BASE PLATE PLAN C2 — TYP ANCHOR ROD

BASE PLATE DETAIL 4

Scale= 1/4" = 1'-0" S-101

SHEARWALL SCHEDULE

TYPE	NAIL SPACING (EN&BN)	HOLDOWN	ANCHOR BOLTS	END POST THICKNESS UON
1	10d @ 4" O.C.	HDU14- SDS2.5	5/8" DIA @ 24" O.C.	6" MIN WOOD END POST THICKNESS
2	10d @ 6" O.C.	HDU8 - SDS2.5	5/8" DIA @ 32" O.C.	4 1/2" MIN WOOD END POST THICKNESS
3	10d @ 4" O.C.	HDU14 - SDS2.5	5/8" DIA @ 24" O.C.	4 1/2" MIN WOOD END POST THICKNESS
4	10d @ 4" O.C.	HDU8 - SDS2.5	5/8" DIA @ 32" O.C.	4 1/2" MIN WOOD END POST THICKNESS

X - DESIGNATES SHEARWALL TYPE

NOTES:

1. USE 10d COMMON NAILS.
2. NAIL PANEL FACES @ 12" O.C. - FN
3. USE 15/32" PLYWOOD.
4. STAGGER PLYWOOD JOINT AND SILL PLATE NAILING.
5. FRAMING MEMBERS OR BLOCKING SHALL BE PROVIDED AT THE EDGES OF ALL SHEETS IN SHEARWALLS.
6. REFER TO 9/S-200 FOR HOLD DOWN ANCHOR EMBEDMENT.
7. HOLD DOWN ANCHORS MUST BE TIED IN PLACE PRIOR TO FOUNDATION INSPECTION.
8. FRAMING AT ADJOINING PANEL EDGES SHALL BE 3 INCH NOMINAL OR THICKER AND NAILS SHALL BE STAGGERED WHERE NAILS ARE SPACED AT 3" O.C. OR ARE ON EACH FACE.
9. FN INDICATES FIELD NAILING
   EN INDICATES EDGE NAILING
   BN INDICATES BOUNDARY NAILING

SHEAR WALL SCHEDULE AND NOTES 3

Scale: NO SCALE S-101

PLAN NOTES

1. REF. SHEET S-000 FOR STRUCTURAL NOTES AND SPECIAL INSPECTIONS.
2. REF. 3/S-201 FOR TYPICAL PIPE DETAIL AS REQUIRED.
3. REF. 4/S-201 FOR TYPICAL SILL PLATE BOLT DETAIL AS REQUIRED.
4. REF. 5/S-201 FOR TYPICAL STEP FOOTING DETAIL AS REQUIRED.
5. REF. 6/S-201 FOR TYPICAL GRADE BEAM STIRRUP DETAIL AS REQUIRED.
6. REF. 7/S-201 FOR TYPICAL REINFORCING BAR DETAIL AS REQUIRED.
7. REF. 8/S-201 FOR TYPICAL CORNER BAR DETAIL AS REQUIRED.
8. REF. 16/S-200 FOR TYPICAL NON BEARING WALL CONNECTION AT SLAB AS REQUIRED.
9. REF. 3/S-301 FOR TYPICAL FRAMING NOTCHES DETAIL AS REQUIRED.
10. REF. 4/S-301 FOR TYPICAL WALL FRAMING DETAILS AS REQUIRED.
11. REF. 12/S-200 FOR TYPICAL TRASH ENCLOSURE DETAIL AS REQUIRED.
12. REF. 11/S-200 FOR TYPICAL LIGHT POLE BASE DETAIL AS REQUIRED.
13. REF 4/S-101 FOR COLUMN BASE PLATE DETAIL.
14. REF TO GEOTECHNICAL REPORT FOR SITE PREPARATION

FOUNDATION NOTES 2

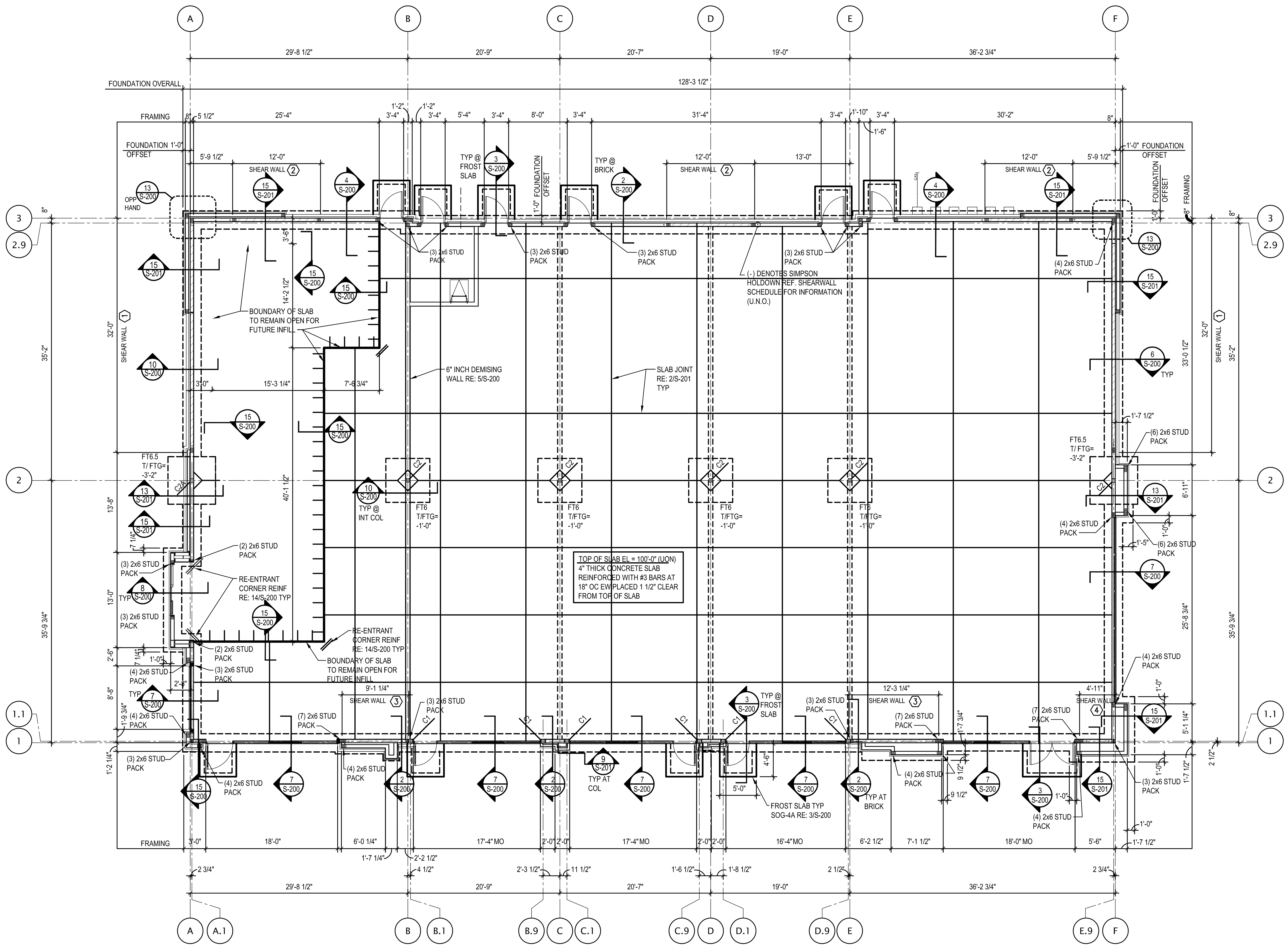
Scale: NO SCALE S-101

STRUCTURAL STEEL COLUMN SCHEDULE

MARK	SIZE	BASE PLATE (t x B x N)	CAP PLATE	ANCHOR RODS
C1	HSS6x6x5/16	3/4" x 12" x 1'-0"		(4) 3/4" DIA
C2, C2A	HSS5 1/2x5 1/2 x3/8	3/4" x 12" x 1'-0"		(4) 3/4" DIA

FOOTING SCHEDULE

MARK	SIZE	REINFORCEMENT
F1	3'-0"x7'-6"x25'-0"	8-#7 LONG BARS T & B #6 x 6'-10" SHORT T & B
FT6	6'-0"x6'-0"x1'-4"	(5) #6 EW TOP & BOT
FT6.5	6'-6"x6'-6"x1'-4"	(6) #6 EW TOP & BOT



FOUNDATION PLAN 1

Scale= 1/8" = 1'-0" S-101

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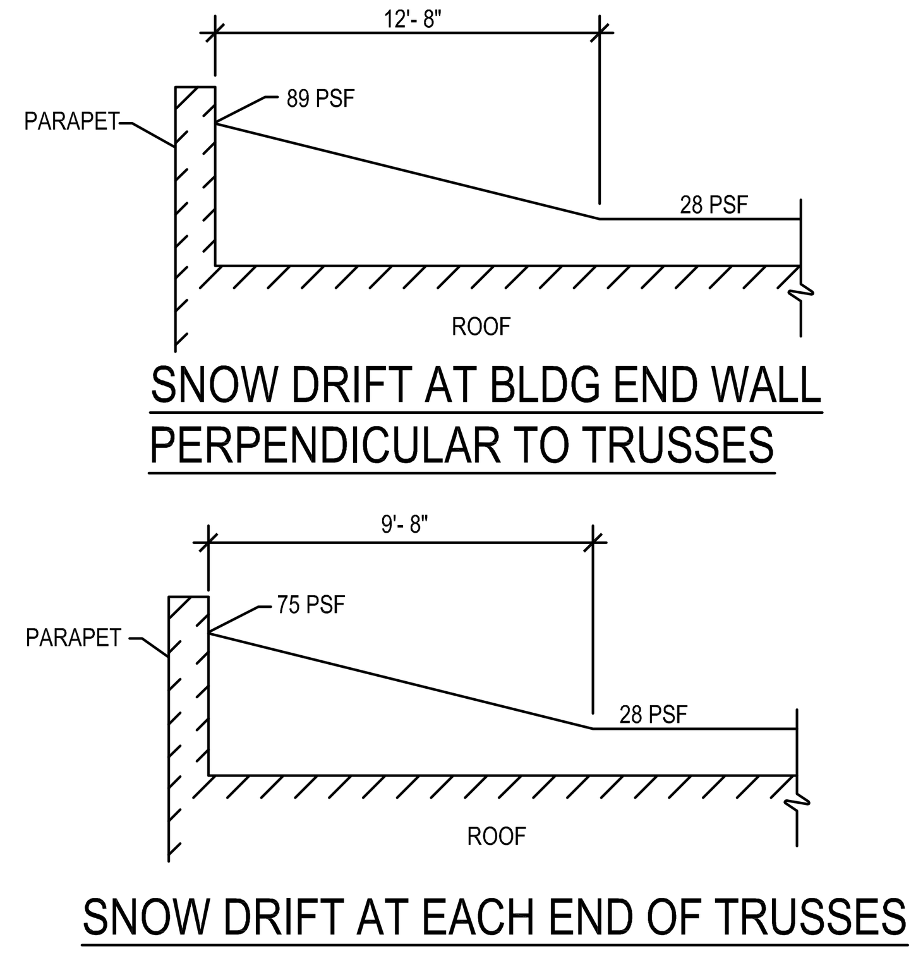
S-101

FOUNDATION PLAN



COMPONENTS AND CLADDING DESIGN WINDLOAD PRESSURE:(ULT)

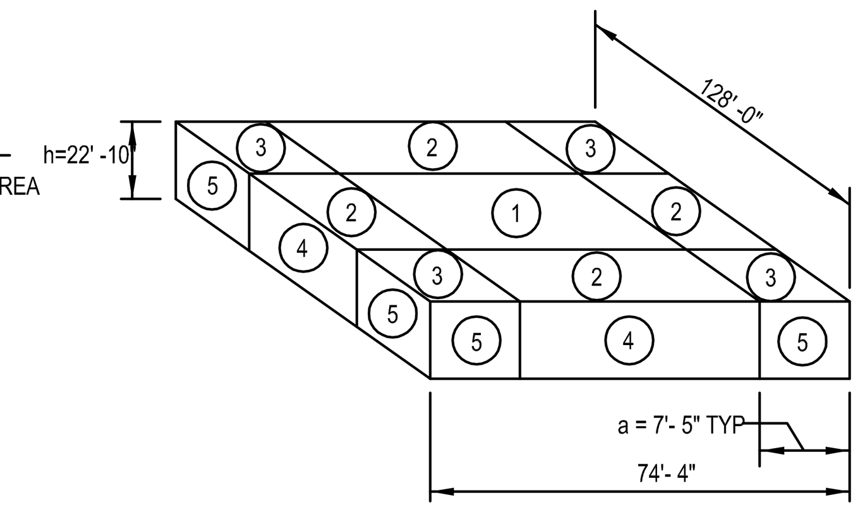
PRESSURES ARE BASED ON 10 SQ FT AREA

ROOF
(1) +12 psf / -29 psf
(2) +12 psf / -49 psf
(3) +12 psf / -49 psf

WALLS
(4) +29 psf / -31 psf
(5) +29 psf / -39 psf

OVERHANG
*-46 psf (UPLIFT)

PLUS AND MINUS SIGNS INDICATE PRESSURES ACTING TOWARD AND AWAY FROM SURFACES, RESPECTIVELY

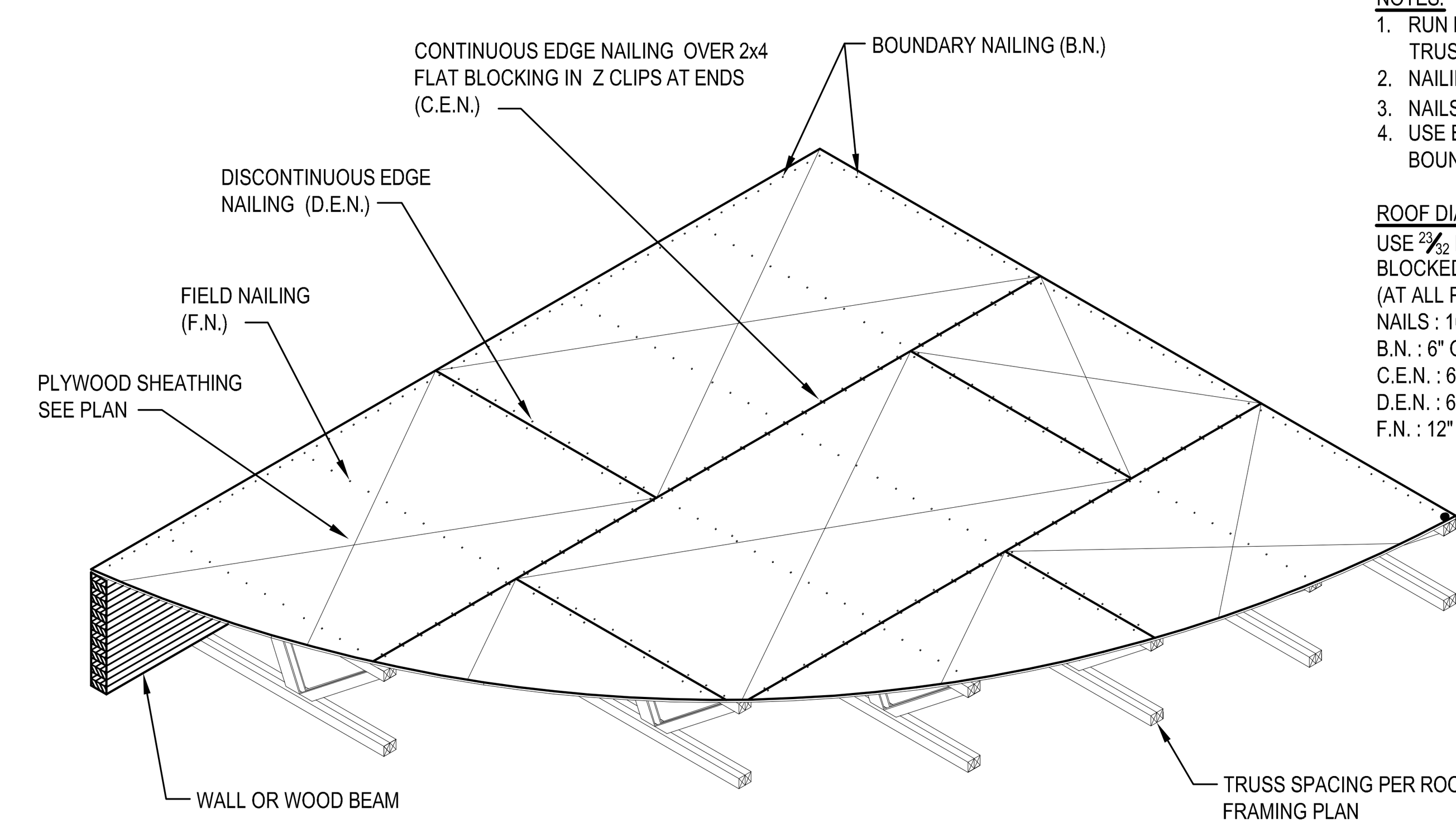


WALL PRESSURE (ULT) (PER ASCE7-10 CHAPTER 27 PART 1)

	WINDWARD	LEEWARD
0'-15'	16.6 PSF	10.4 PSF
20.0'	17.6 PSF	10.4 PSF

WIND NET UPLIFT (ASD) (PSF)			
ROOF DECK		ROOF TRUSSES	
TYPICAL	W/IN 7.43' OF PERIMETER	TYPICAL	W/IN 7.43' OF PERIMETER
17	28	12	16

TYPICAL LOAD DIAGRAM DETAIL 4
S-102



NOTES:
1. RUN LONG DIMENSION OF PLYWOOD PERPENDICULAR TO TRUSSES.
2. NAILING SIZE AND SPACING AS NOTED ON PLAN.
3. NAILS SHALL HAVE A MIN. 3/8" EDGE DISTANCE.
4. USE BOUNDARY NAILING CONTINUOUS @ ALL NAILING ZONE BOUNDARIES. SEE PLAN FOR ZONE EXTENTS.

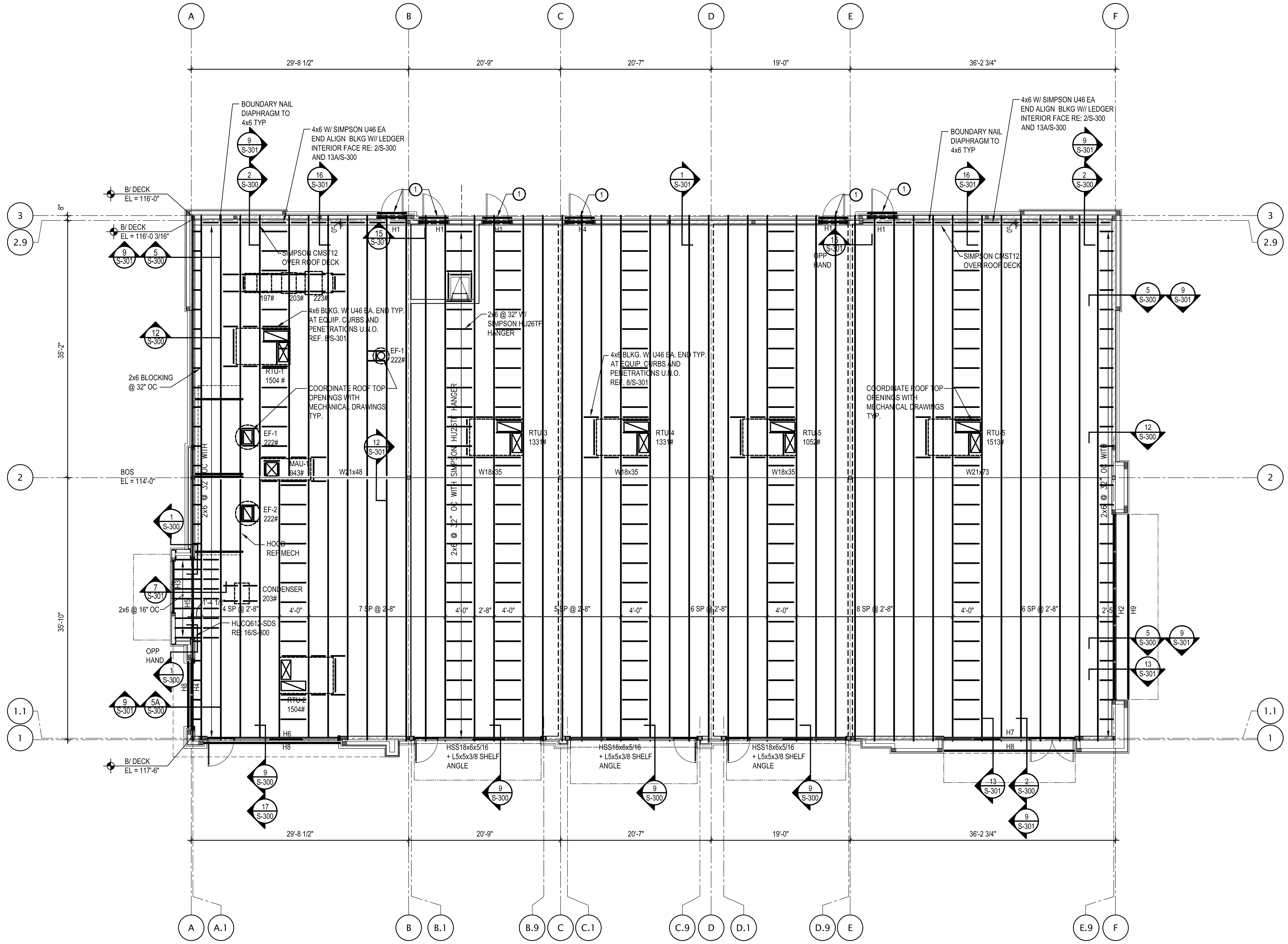
ROOF DIAPHRAGM
USE 23/32 PLYWOOD, INDEX 48/24
BLOCKED DIAPHRAGM 2x4 FLAT IN Z CLIP
(AT ALL PLYWOOD EDGES, TYP. U.N.O.)
NAILS : 10d COMMON NAILS
B.N. : 6" O.C.
C.E.N. : 6" O.C.
D.E.N. : 6" O.C.
F.N. : 12" O.C.

TYPICAL ROOF DIAPHRAGM DETAIL 3
S-102

PLAN NOTES

1. REF. SHEET S-000 FOR STRUCTURAL NOTES AND SPECIAL INSPECTIONS.
2. REF. 9/S-301 FOR TYPICAL SUSPEND THREADED ROD SUPPORT DETAIL AS REQUIRED.
3. REF. 5/S-301 FOR BLOCKING AT CANOPY. CANOPY BY OTHERS
4. REF. 2/S-301 FOR TYPICAL HEADER DETAIL AS REQUIRED U.N.O.
5. REF. 2/S-301 FOR TYPICAL MECHANICAL UNIT SUPPORT DETAIL AS REQUIRED.
6. REF. 6/S-301 FOR TYPICAL BRIDGING AND BRACING DETAIL AS REQUIRED.
7. REF. 10/S-301 FOR TYPICAL PARTITION WALL SUPPORT PARALLEL TO TRUSS DETAIL AS REQUIRED.
8. REF. 11/S-301 FOR TYPICAL PARTITION WALL SUPPORT PERPENDICULAR TO TRUSS DETAIL AS REQUIRED.
9. REF. 3/S-101 FOR SHEARWALL SCHEDULE.
10. REF 19/S-301 FOR HEADER SCHEDULE
11. ALL EXTERIOR STUD WALLS SHALL HAVE 1/2" WOOD SHEATHING ON THE OUTSIDE FACE (BOTH FACES ABOVE PARAPET) REFER TO 3/S-101 FOR SHEATHING REQUIREMENTS AT SHEARWALLS REFER TO ARCH.
12. REF 19/S-300 FOR HEADER SCHEDULE

TYPICAL ROOF NOTES 2
S-102



KEYNOTES:
(1) L5x5x3/8 LOOSE LINTEL 8" BEARING EA END

ROOF FRAMING PLAN 1
Scale= 1/8" = 1'-0" S-102

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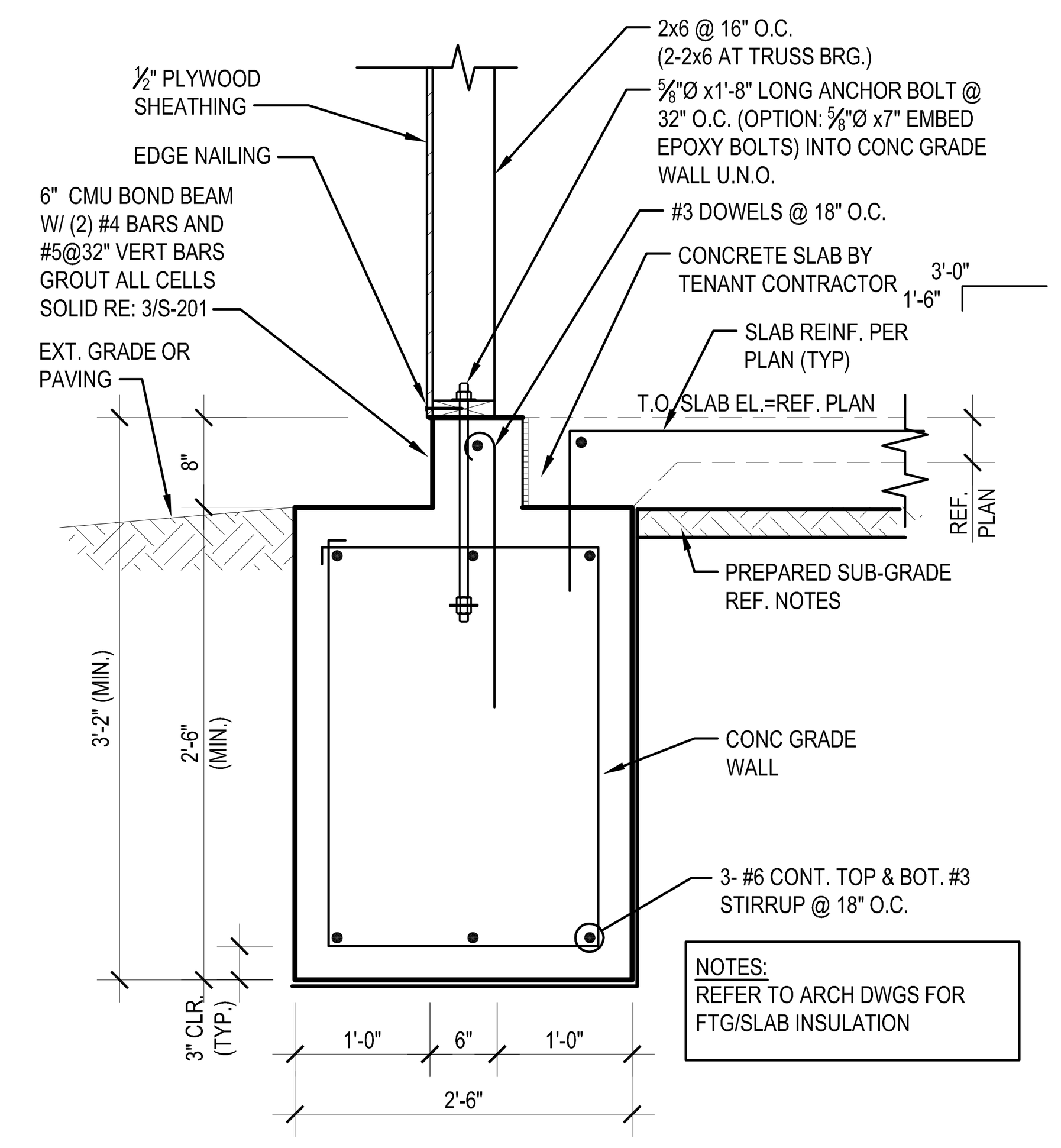
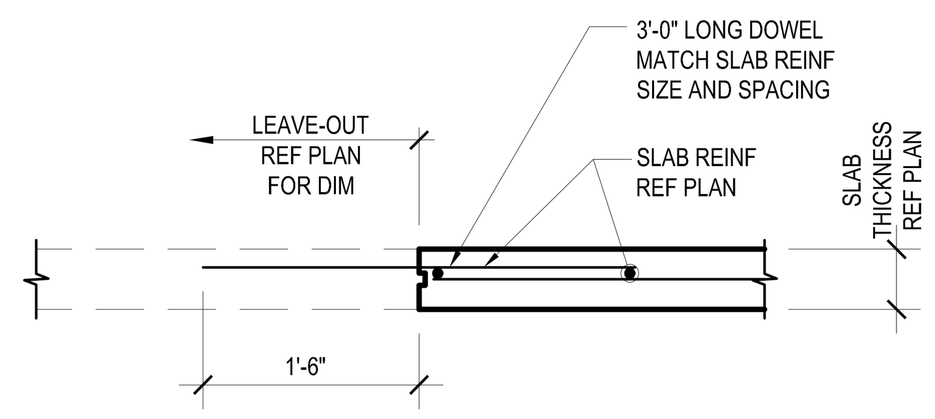
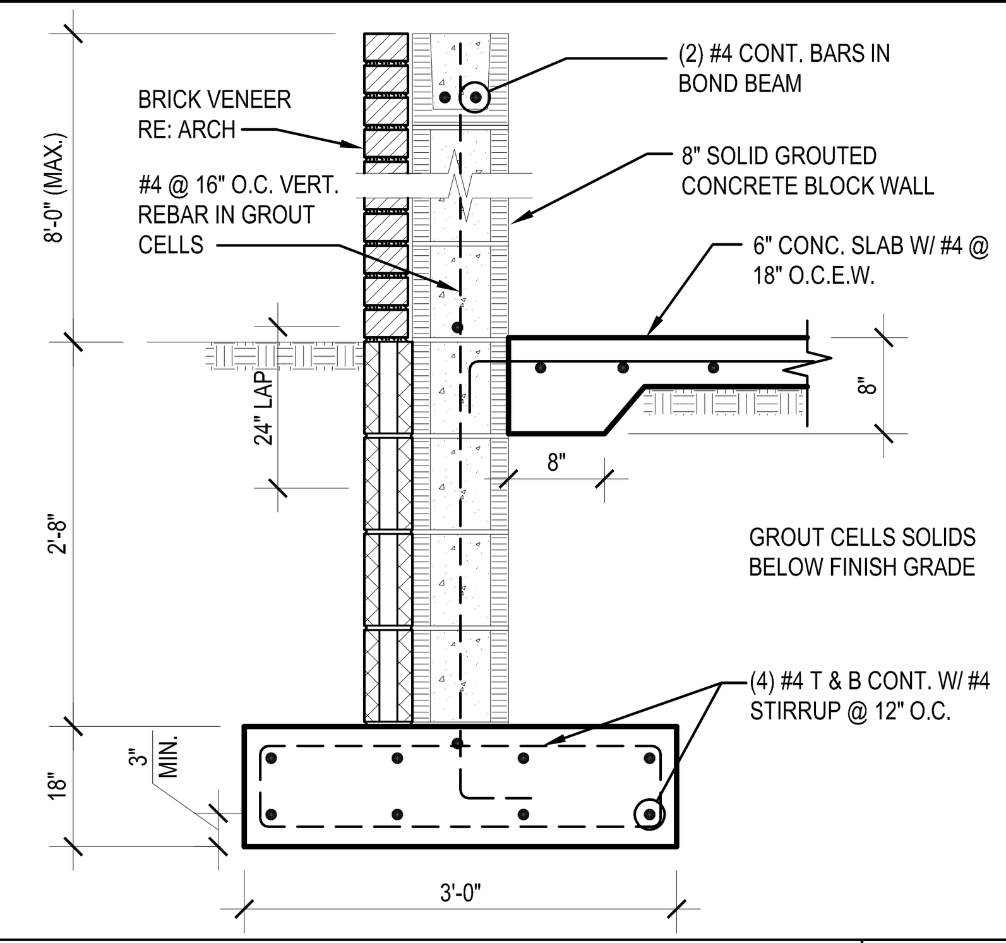
S-102
ROOF FRAMING PLAN



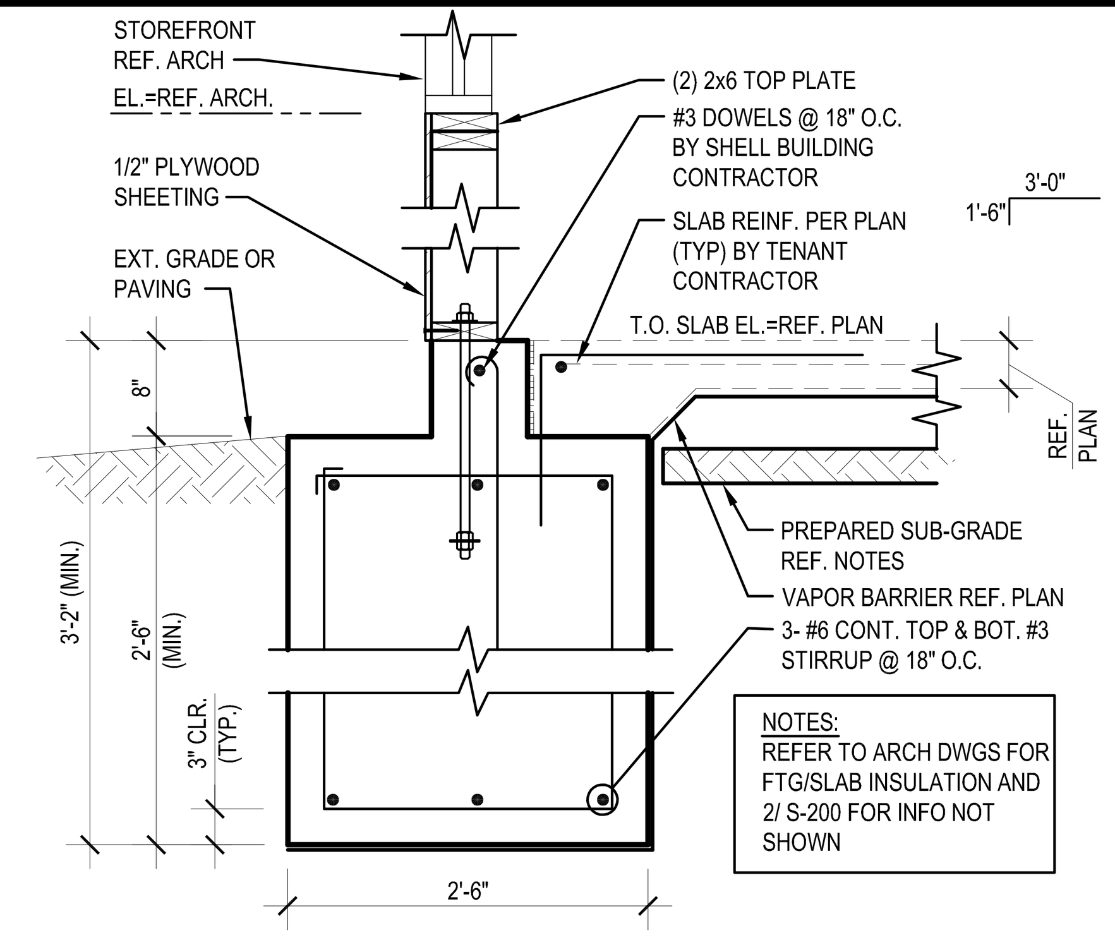
SECTION 19 S-200



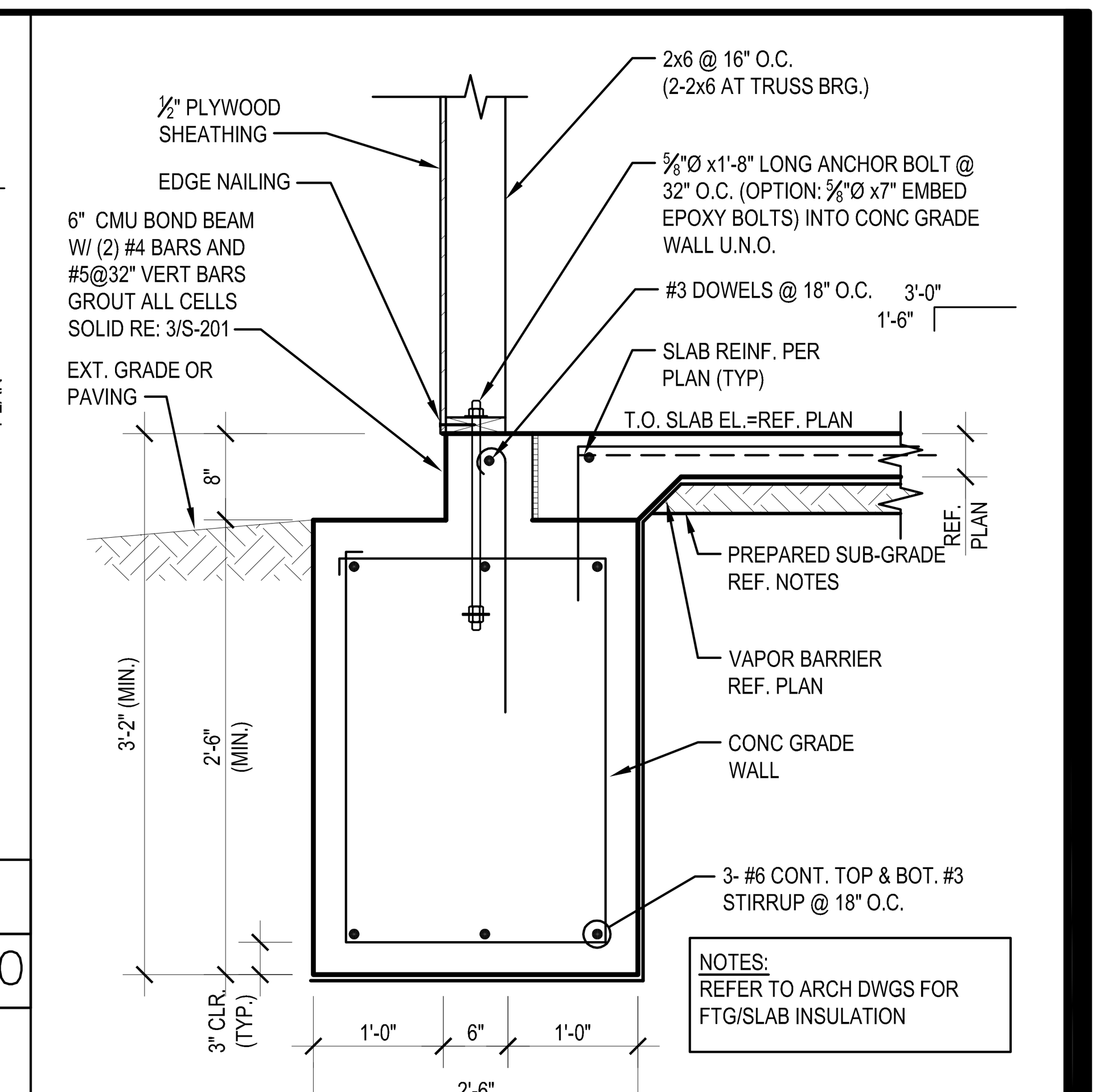
SLAB LEAVE-OUT 15 S-200



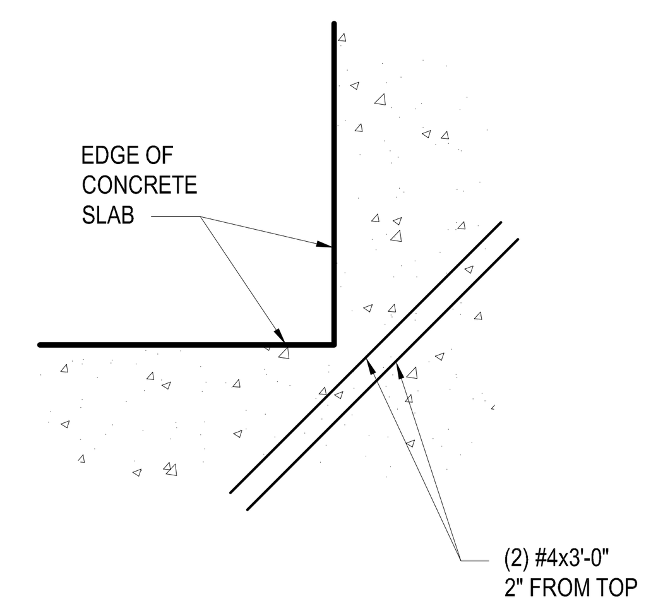
TRASH ENCLOSURE WALL 12 SCALE: 1/2"=1'-0" S-200



DRIVE THRU 8 SCALE: 1/2"=1'-0" S-200

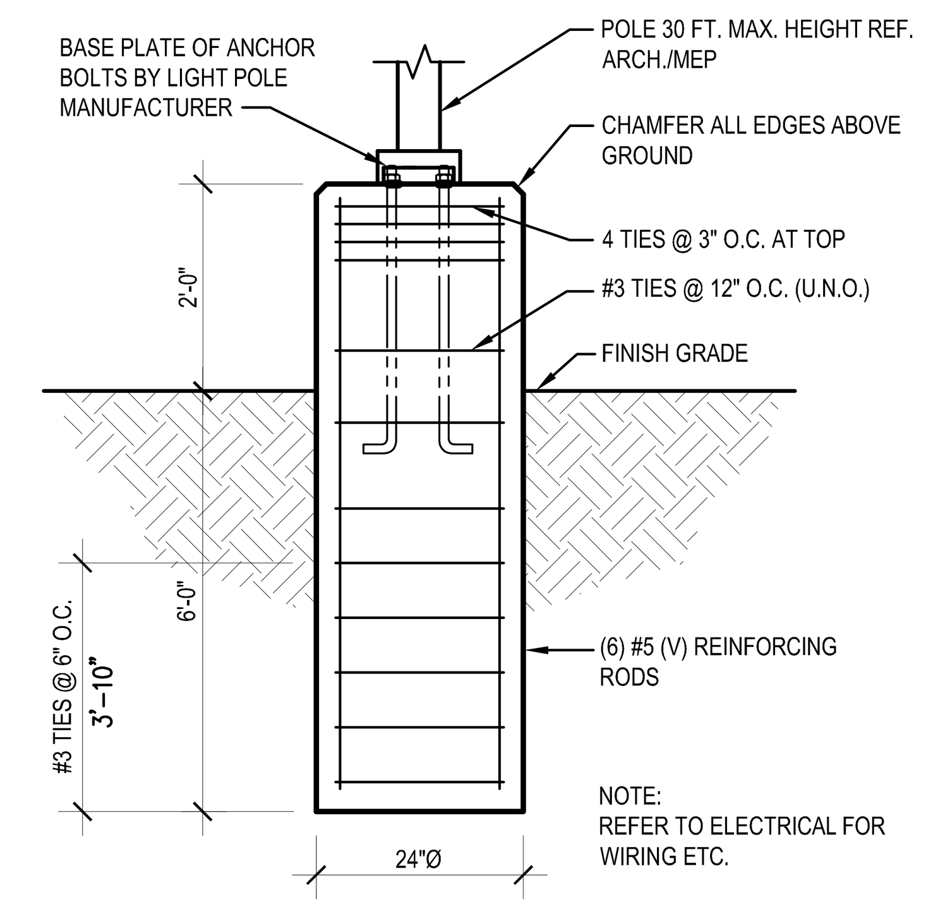


SECTION 4 SCALE: 1/2"=1'-0" S-200

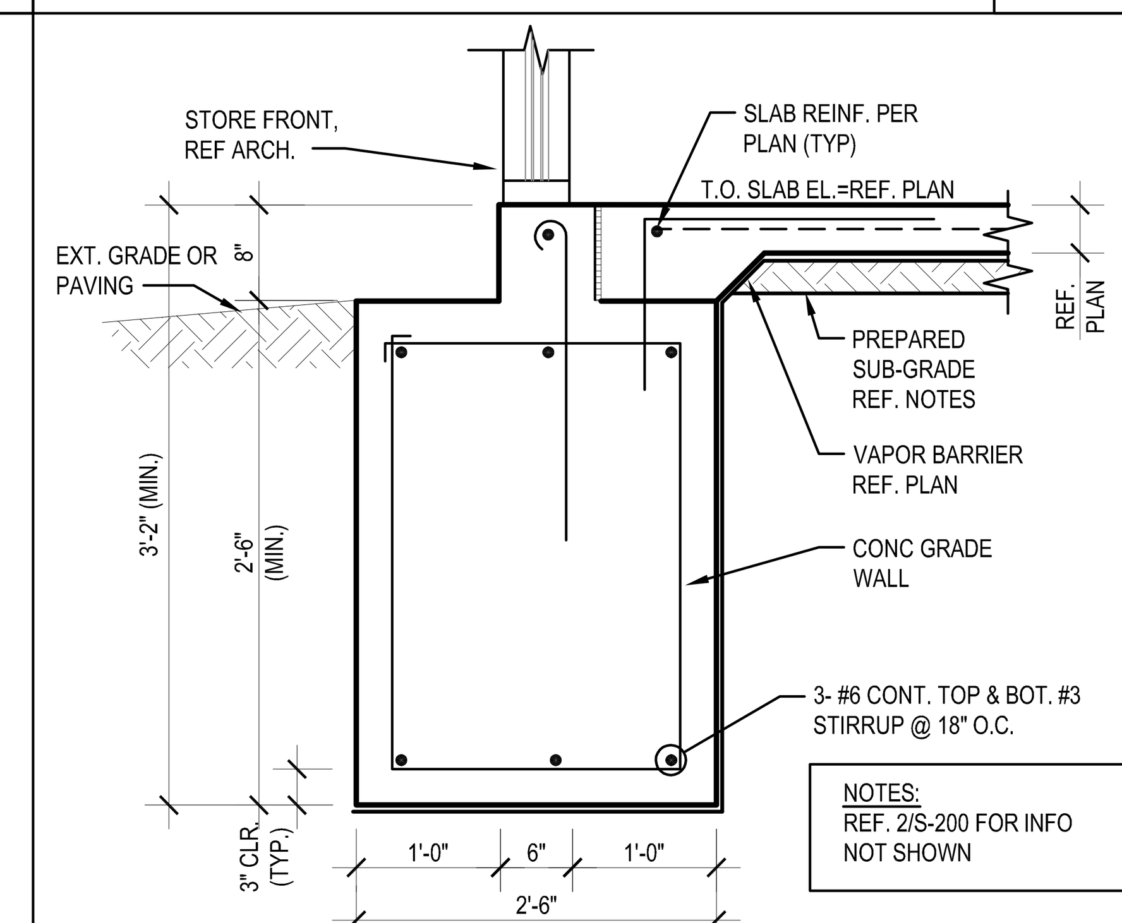


NOT USED 18 S-200

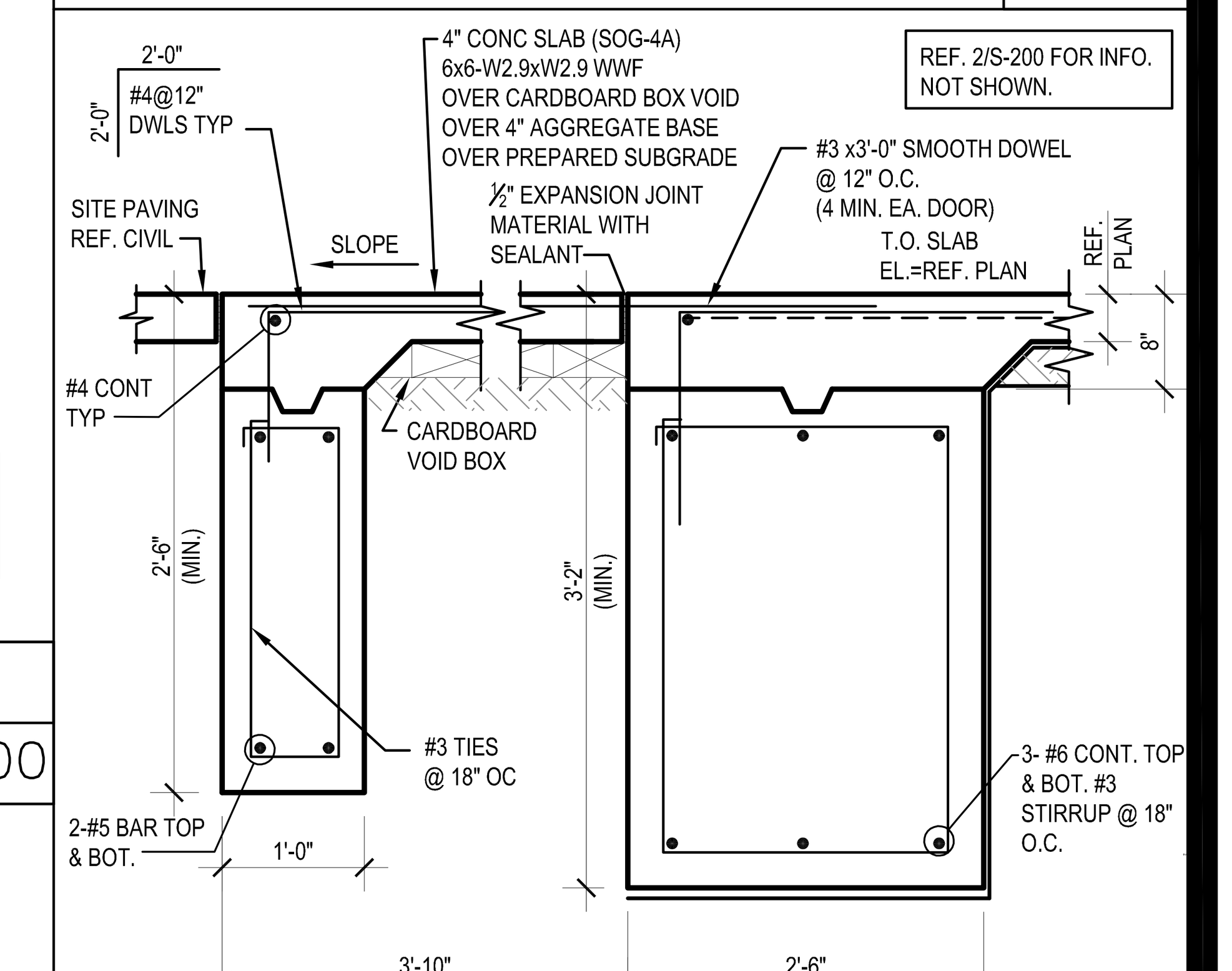
RE-ENTRANT DETAIL 14 SCALE: 1/2"=1'-0" S-200



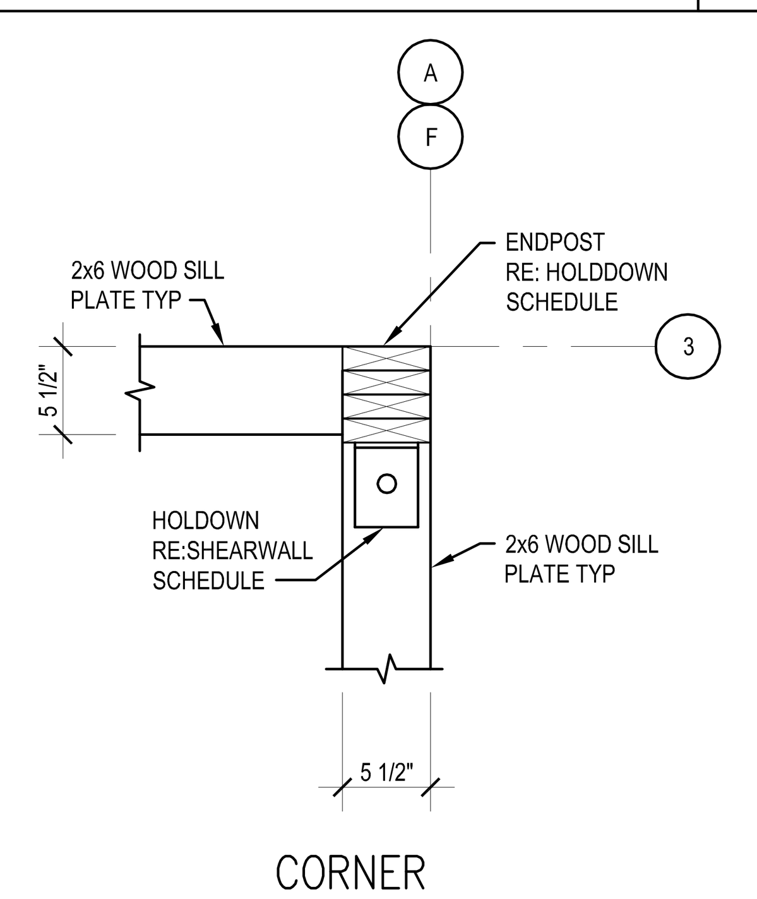
LIGHT POLE BASE 11 SCALE: 1/2"=1'-0" S-200



SECTION 7 S-200

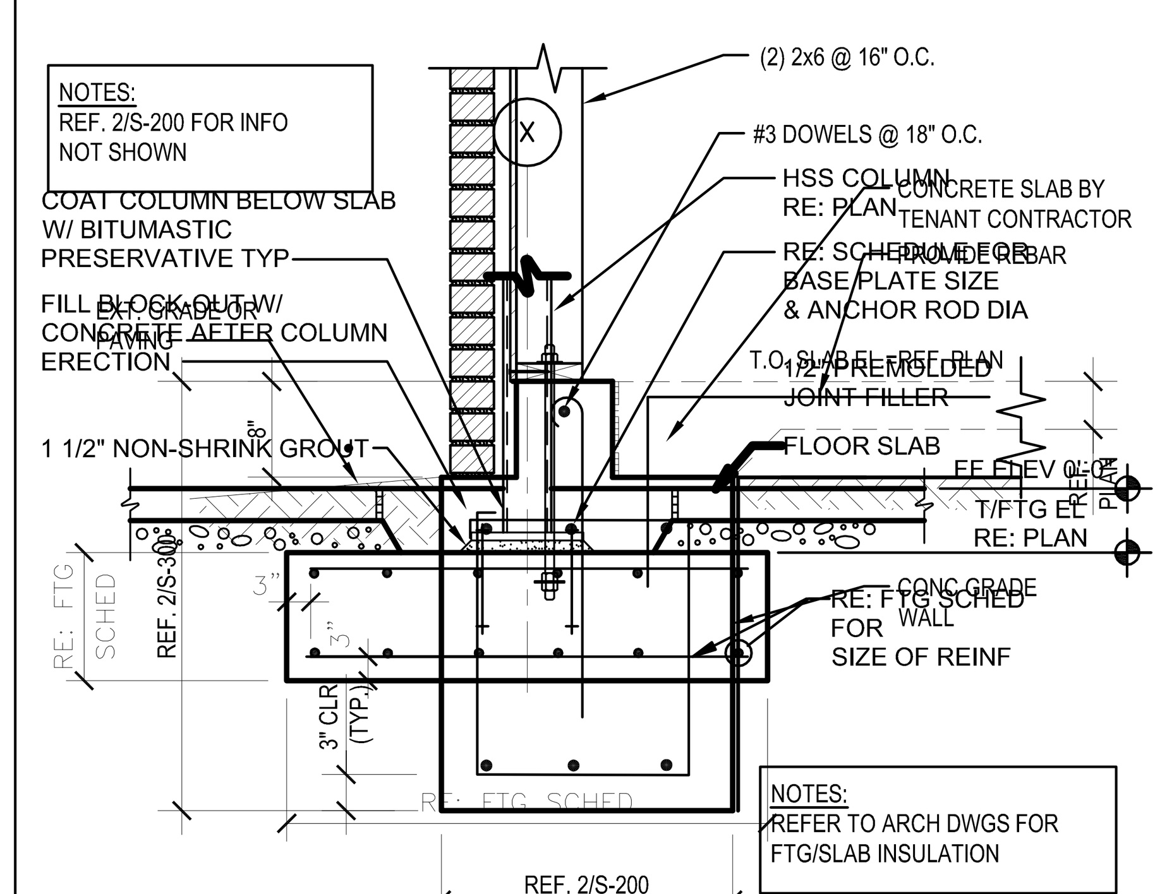


SECTION 3 SCALE: 1/2"=1'-0" S-200

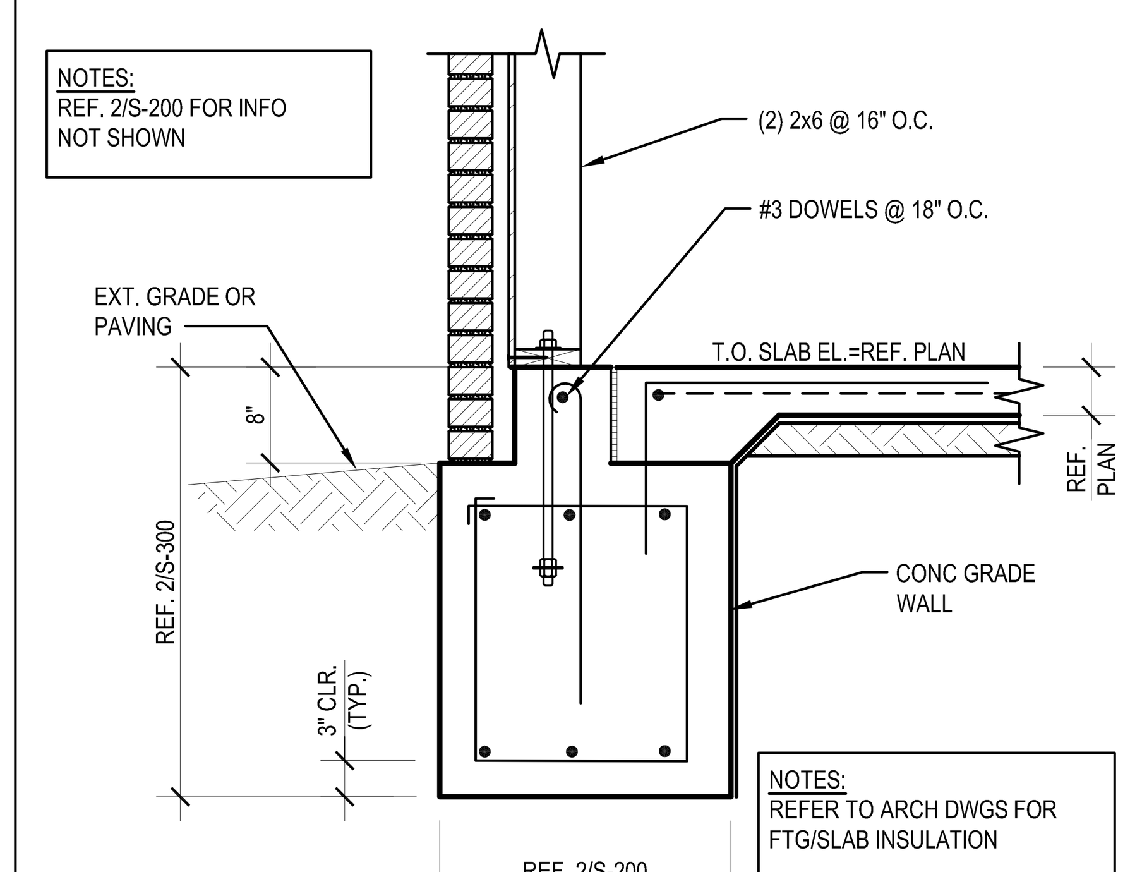


NOT USED 17 SCALE: N.T.S. S-200

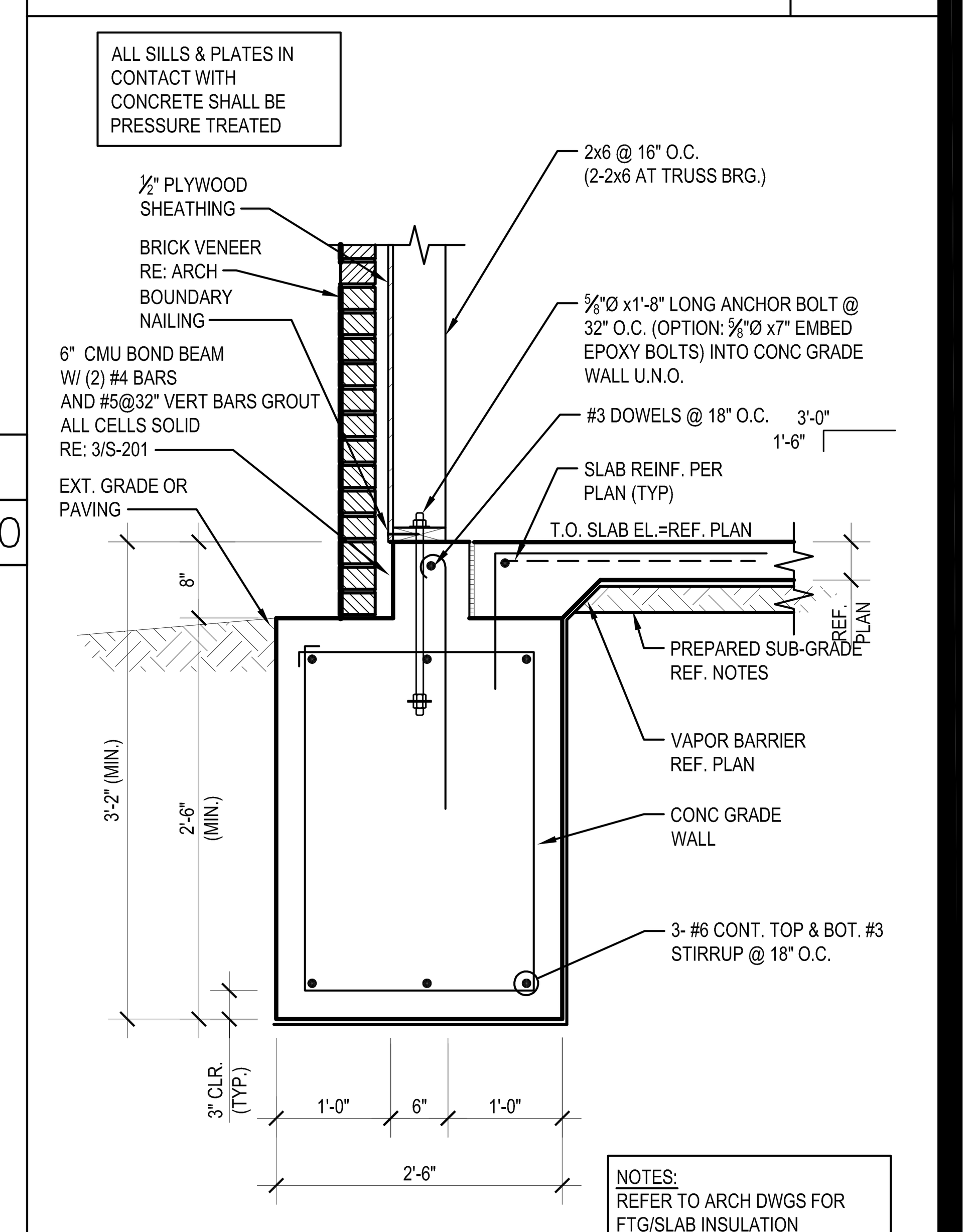
CORNER HOLDOWN DETAIL 13 SCALE: N.T.S. S-200

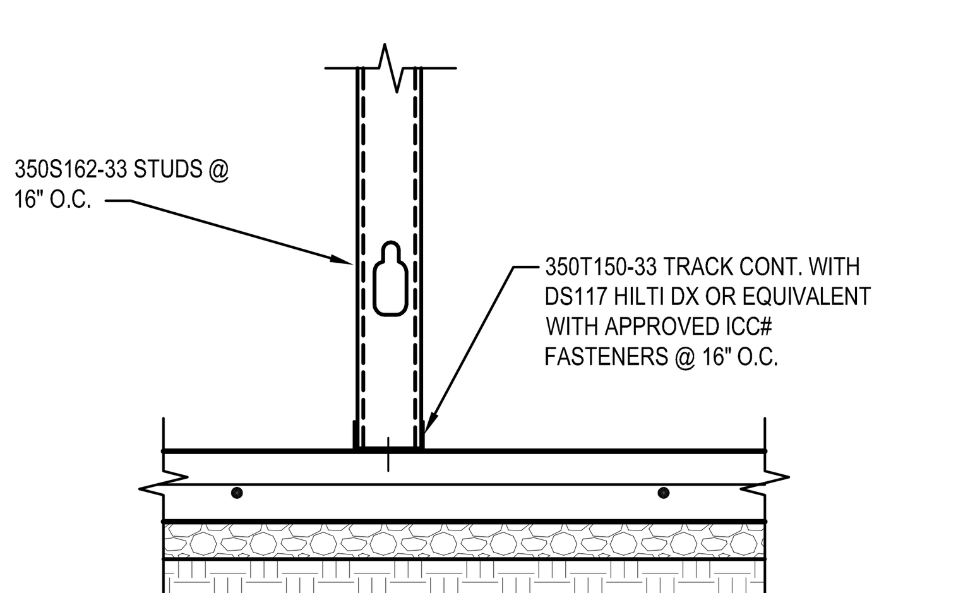


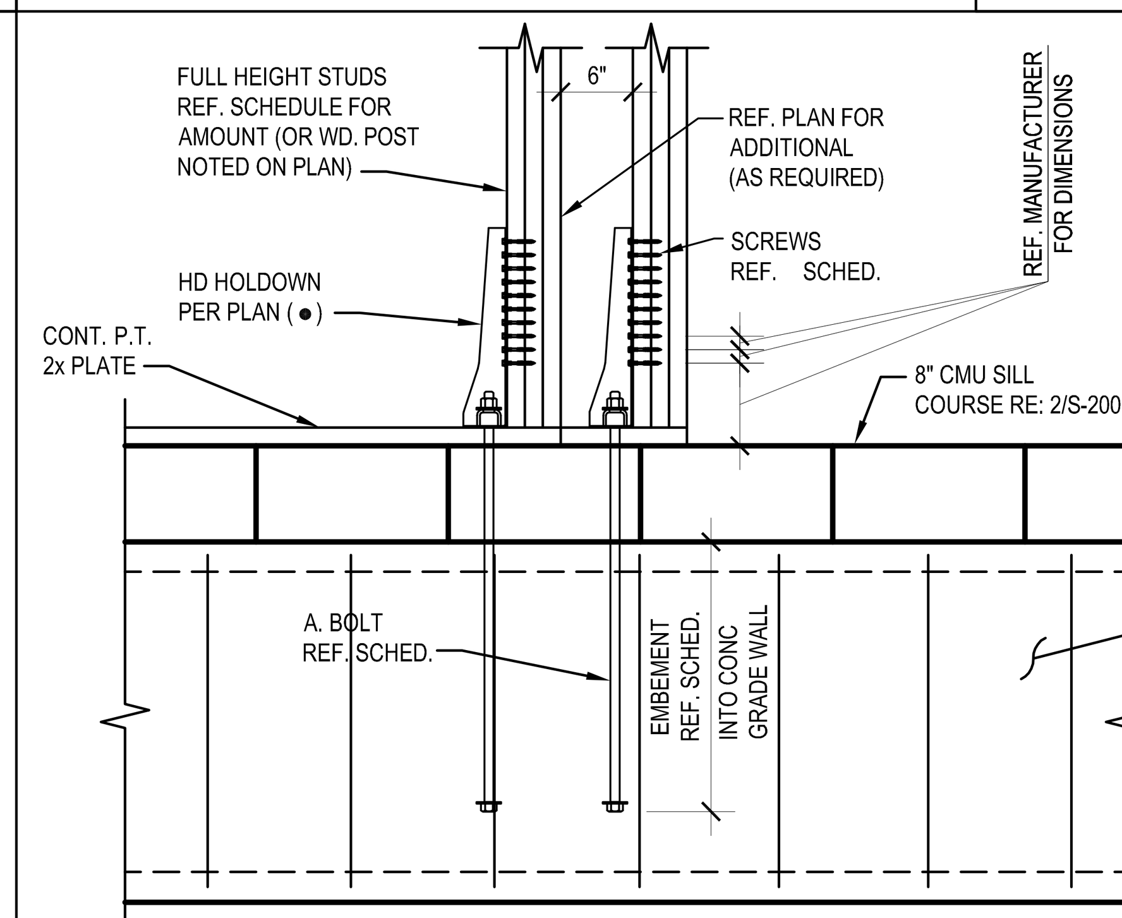
SECTION 10 SCALE: N.T.S. S-200



SECTION 6 SCALE: 1/2"=1'-0" S-200



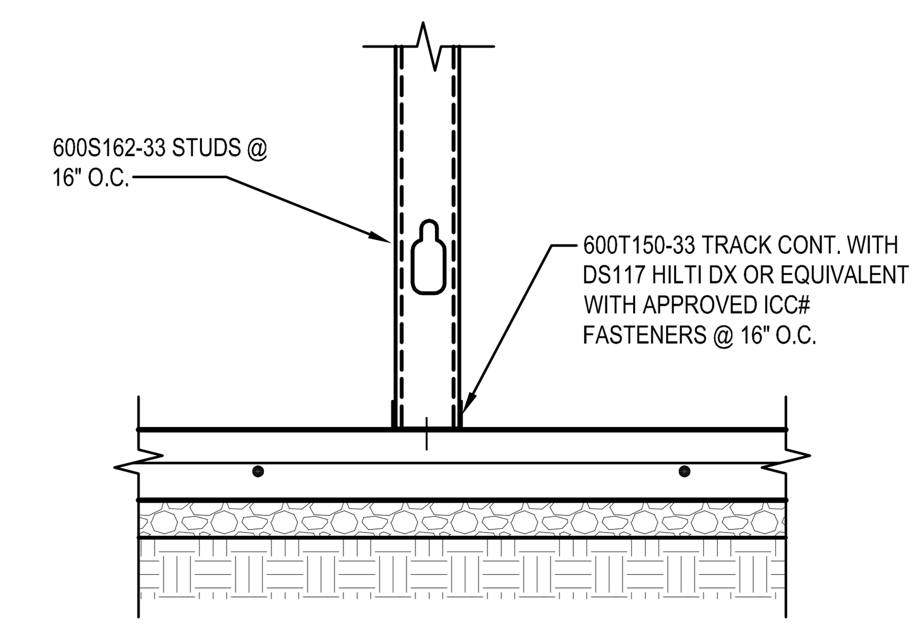




CONT. P.T. 2x PLATE

HOLDOWN	STUDS	SCREW #	CAST IN PLACE REQUIREMENTS ANCH. BOLT	CAST IN PLACE REQUIREMENTS BOLT EMBED	EPOXY OPTION REQUIREMENTS BOLT EMBED
HDU2-SDS2.5	3	6- SDS 1/4"x2 1/2"	5/8"Ø	1'-6"	10"
HDU5-SDS2.5	3	14- SDS 1/4"x2 1/2"	5/8"Ø	1'-6"	12"
HDU8-SDS2.5	3	20- SDS 1/4"x2 1/2"	7/8"Ø	1'-6"	12"
HDU11-SDS2.5	4	30- SDS 1/4"x2 1/2"	1"Ø	2'-0"	16"
HDU14-SDS2.5	4	36- SDS 1/4"x2 1/2"	1"Ø	2'-0"	20"

NOTE: EPOXY OPTION IS BASED ON SIMPSON SET-XP DESIGN VALUES. EPOXY SUBSTITUTIONS MUST BE APPROVED BY STRUCTURAL ENGINEER OF RECORD BEFORE PLACEMENT.



SLAB AT NON BEARING WALL 16 SCALE: 1/2"=1'-0" S-200

TYPICAL HOLDOWN DETAIL- WOOD 9 SCALE: 1/2"=1'-0" S-200

DEMISING WALL 5 SCALE: 1/2"=1'-0" S-200

SECTION 2 SCALE: 1/2"=1'-0" S-200

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CFT PROJECT #:

ARCH PROJECT #: JCDT190143

STATE OF MARYLAND PROFESSIONAL ENGINEER No. 38483

11-13-20

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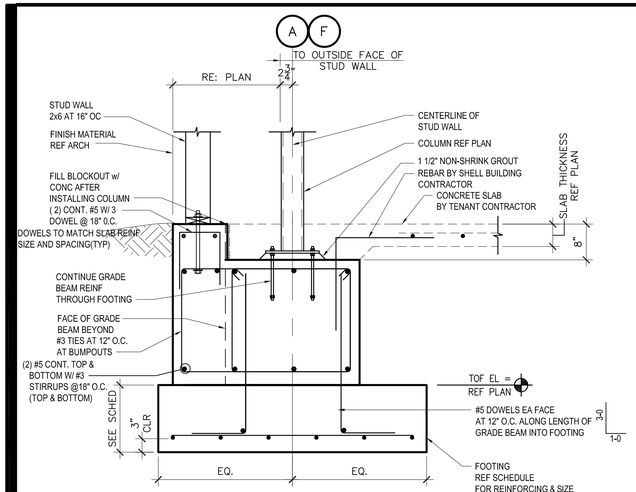
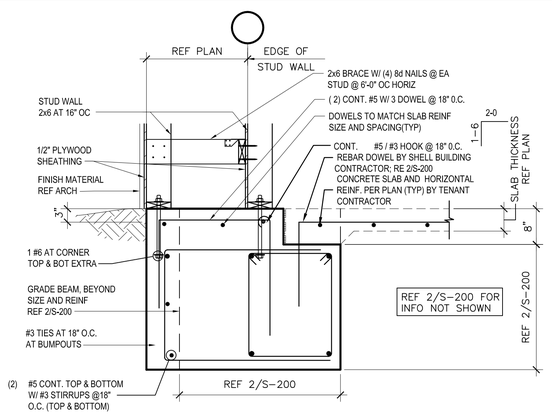
S-200

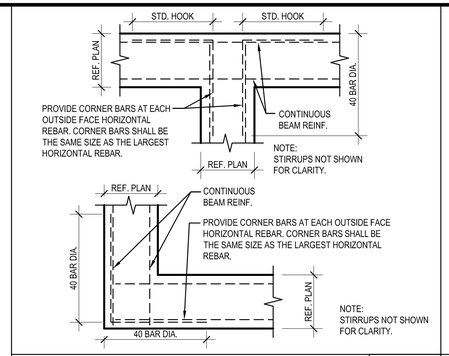
FOUNDATION DETAILS



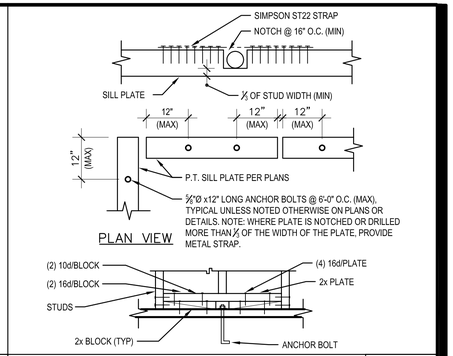
SECTION	20
	S-201



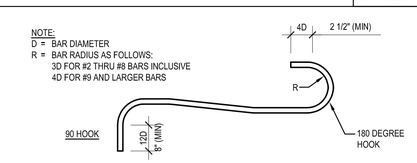
SECTION	11
	S-201



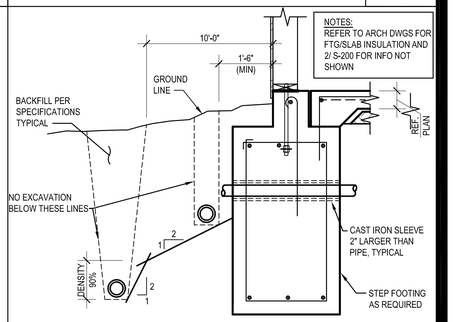
TYPICAL CORNER BAR	8
SCALE: N.T.S.	S-201



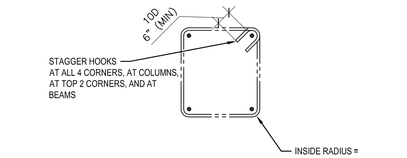
TYPICAL SILL PLATE BOLTS	4
SCALE: N.T.S.	S-201



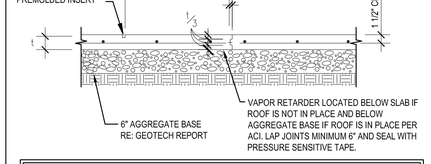
TYPICAL REINFORCING BAR	7
SCALE: N.T.S.	S-201



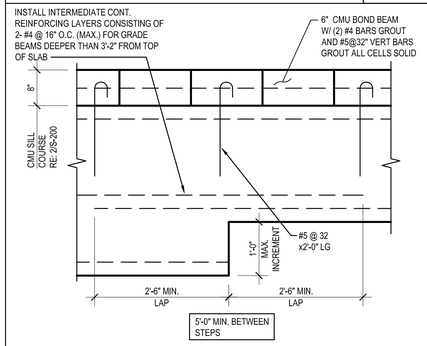
TYPICAL PIPE & TRENCH DETAIL	3
SCALE: N.T.S.	S-201



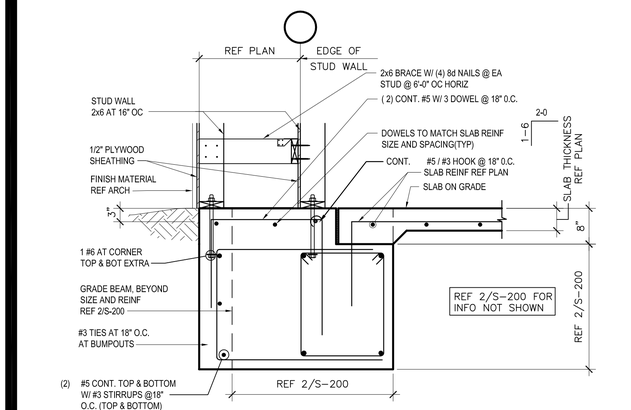
TYPICAL TIES & STIRRUPS	6
SCALE: N.T.S.	S-201



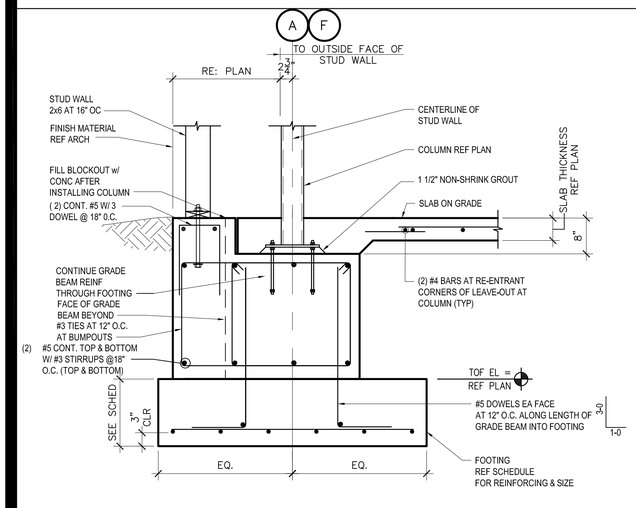
TYPICAL JOINT DETAIL	2
SCALE: ½"=1'-0"	S-201



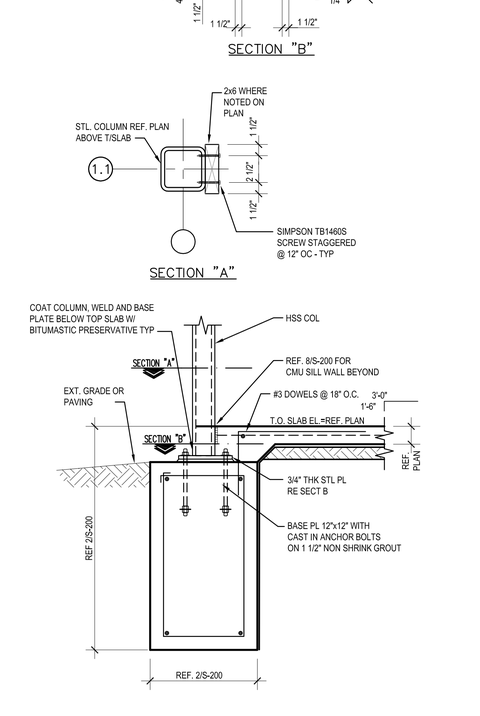
TYPICAL STEP FOOTING	5
SCALE: N.T.S.	S-201



SECTION AT PILASTER & BUMPOUT	15
SCALE: N.T.S.	S-201



FOOTING SECTION AT COLUMN	13
SCALE: ½"=1'-0"	S-201



SECTION	9
SCALE: ½"=1'-0"	S-201

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S-201

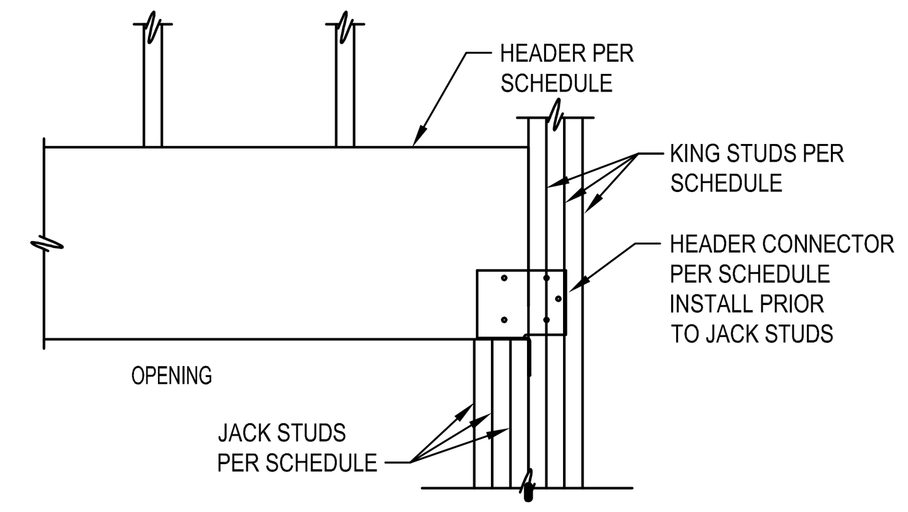
TYPICAL FOUNDATION DETAILS

HEADER SCHEDULE

HEADER MARK	SCHEDULE TYPE	CONNECTOR	JACK STUDS	KING STUDS
H1	(3) 2x6 W/ 1/2" PLYWOOD SPACERS	SIMPSON HH6	(1) 2x6	(2) 2x6
H2	5 1/2x21 24F–V8 GLB	SIMPSON HH6	(2) 2x6	(2) 2x6
H3	5 1/2x12 24F–V8 GLB	SIMPSON HH6	(2) 2x6	(2) 2x6
H4	(3) 2x12 w/ 1/2" PLYWOOD SPACERS	SIMPSON HH6	(1) 2x6	(2) 2x6
H5	4x10	SIMPSON HH6	—	(2) 2x6
H6	5 1/2x24 24F–V8 GLB	SIMPSON HH6	(4) 2x6	(3) 2x6
H7	5 1/2x22 1/2 24F–V8 GLB	SIMPSON HH6	(4) 2x6	(3) 2x6
H8	5 1/2x19 1/2 24F–V8 GLB	SIMPSON HH6	(2) 2x6	(2) 2x6
H9	5 1/2x22 1/2 24F–V8 GLB	SIMPSON HH6	(2) 2x6	(2) 2x6

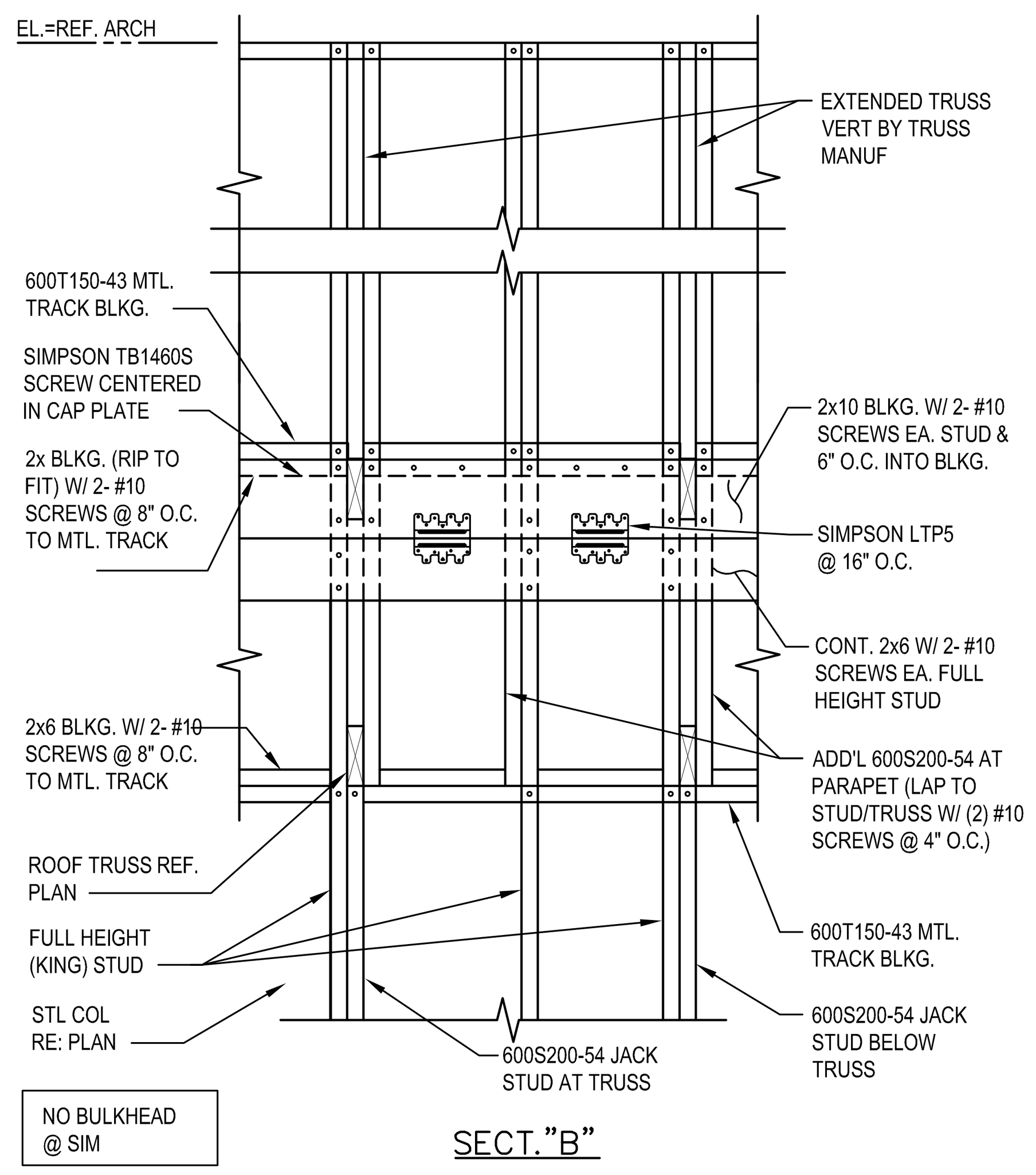
WOOD HEADER SCHEDULE 19

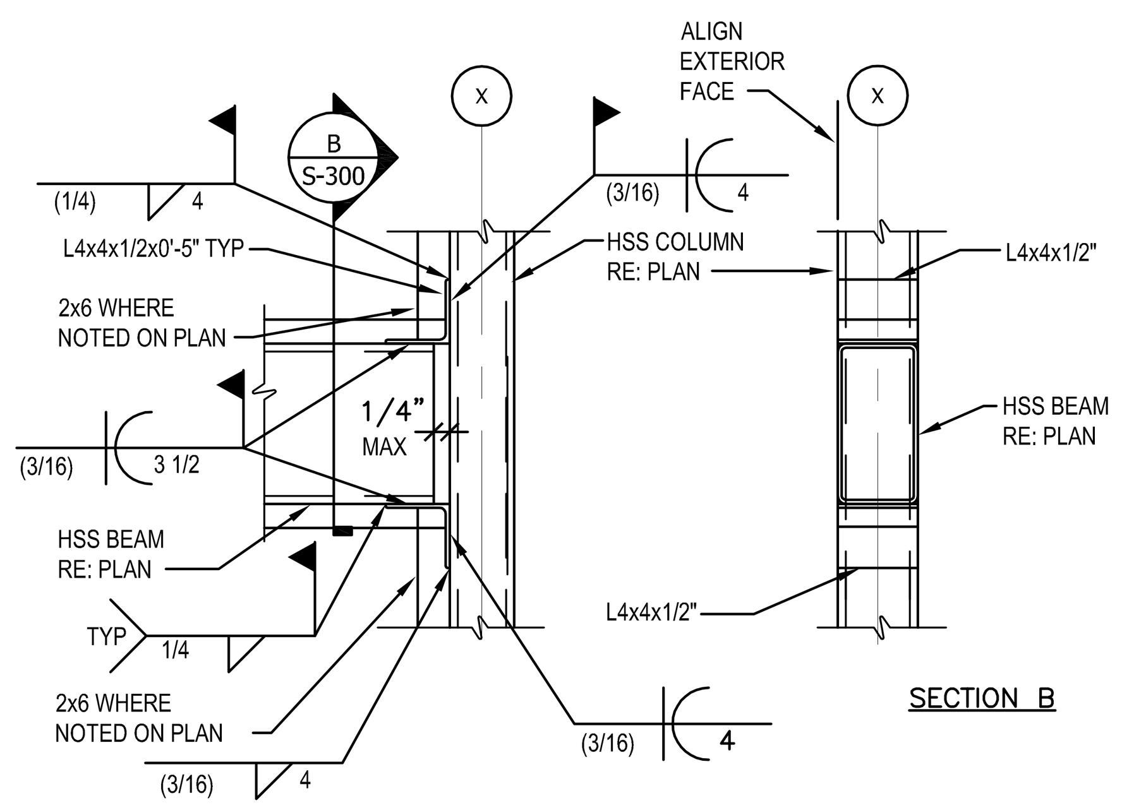
SCALE: 3/4"=1'-0" S–300



WOOD HEADER SECTION 16

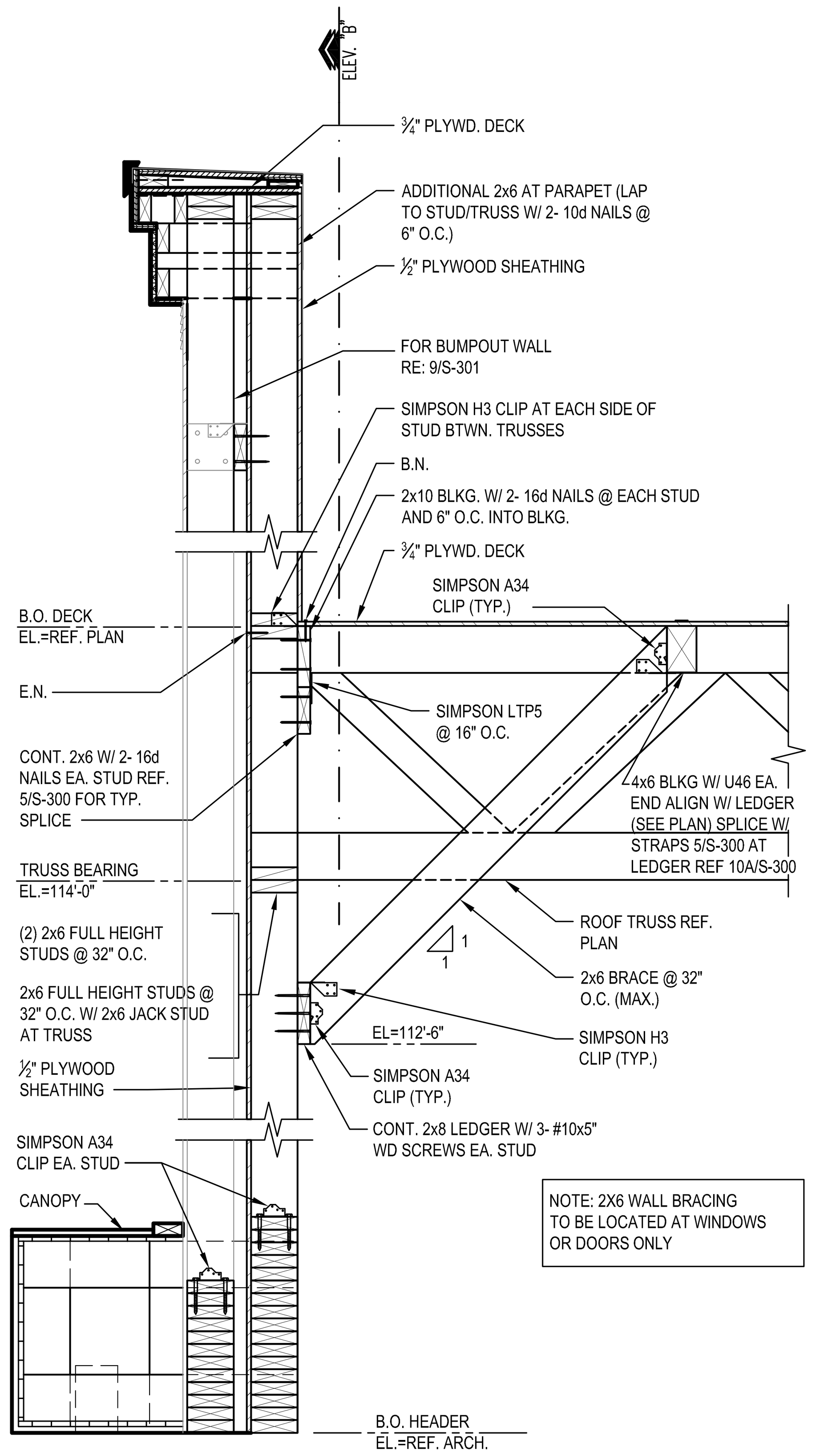
SCALE: 3/4"=1'-0" S–300





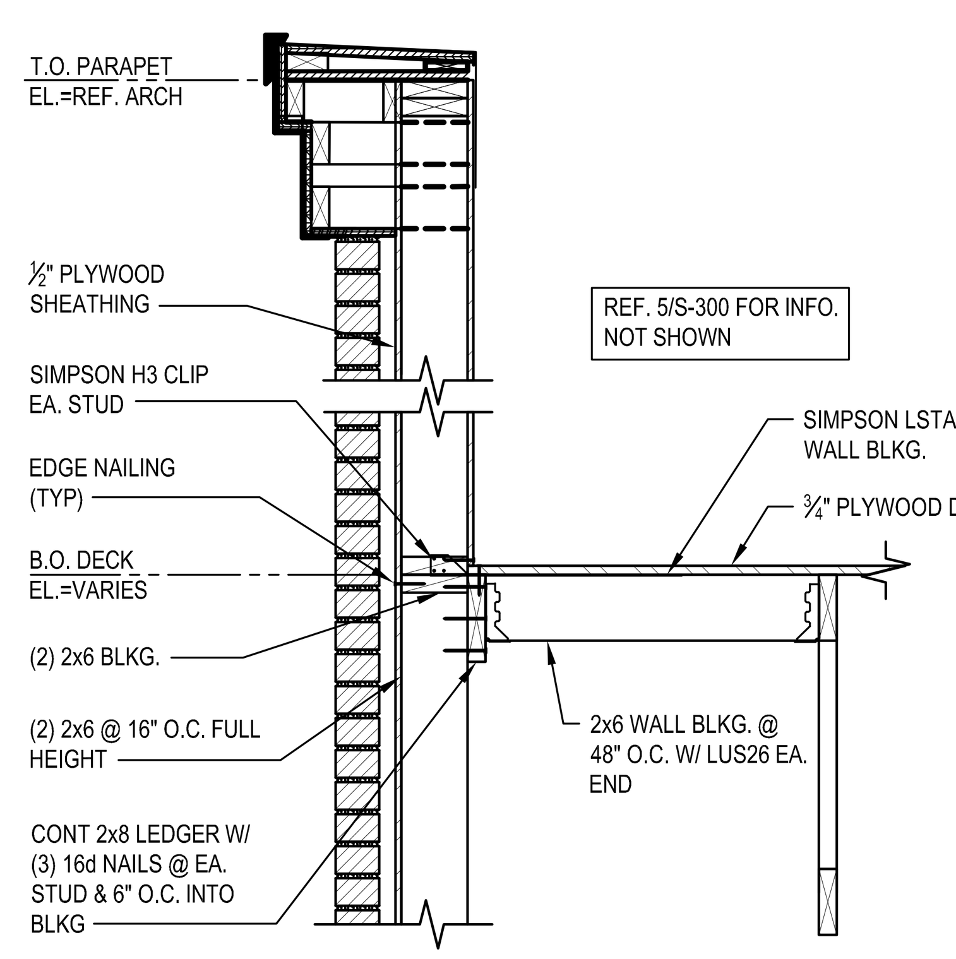
HSS BEAM TO COLUMN CONN 15

SCALE: 3/4"=1'-0" S–300



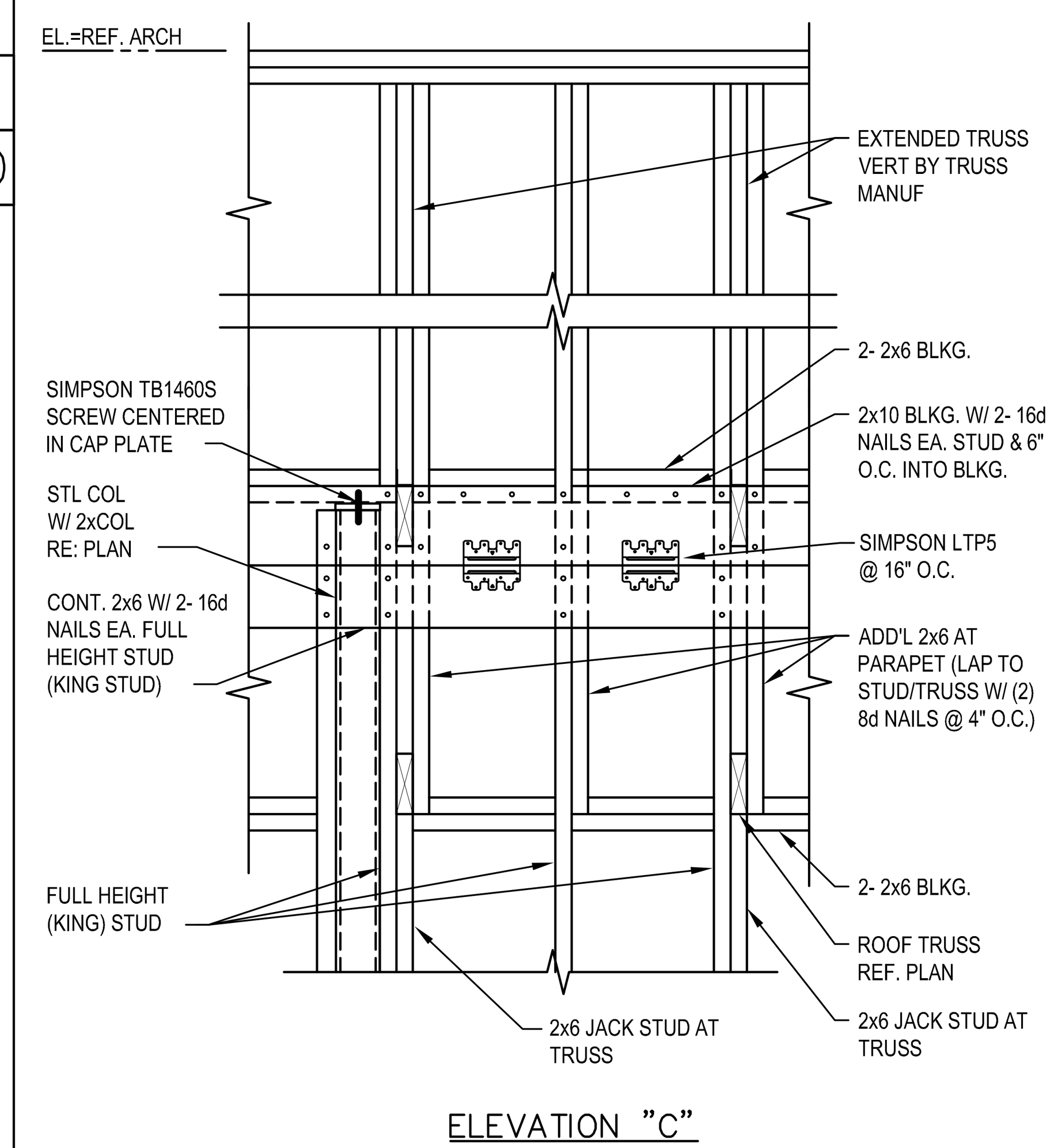
SECTION 17

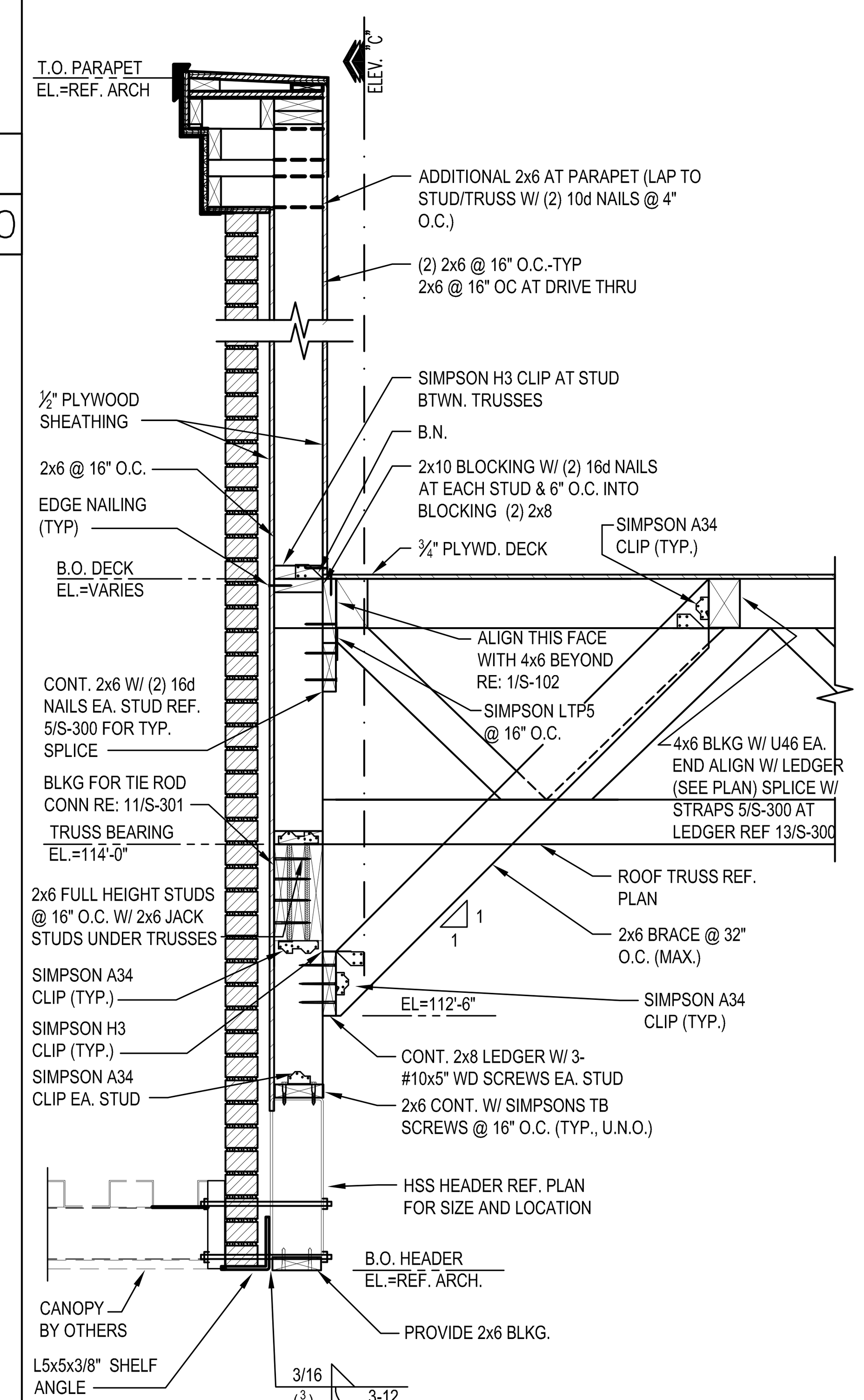
SCALE: 3/4"=1'-0" S–300



SECTION 12

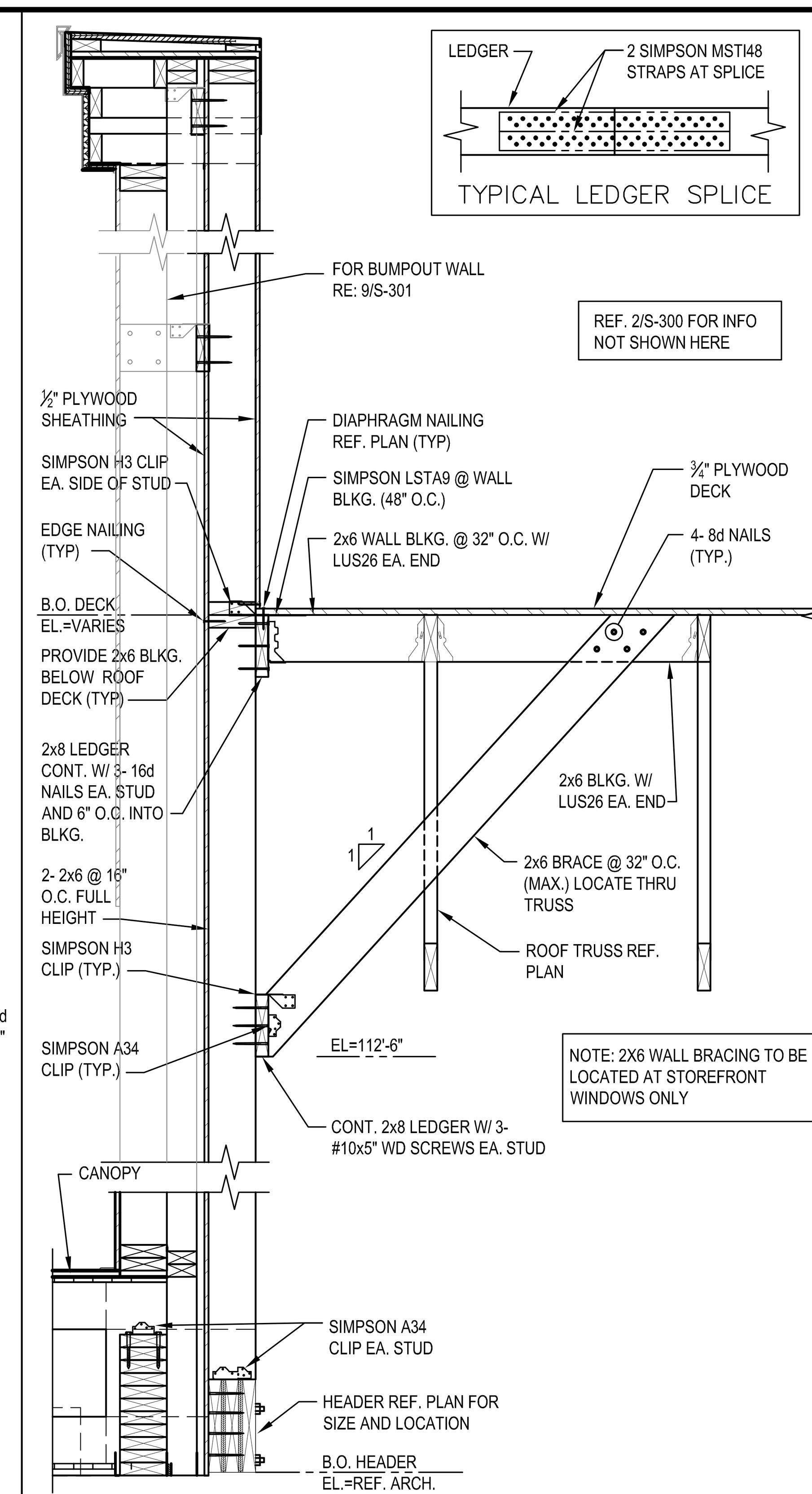
SCALE: 3/4"=1'-0" S–300





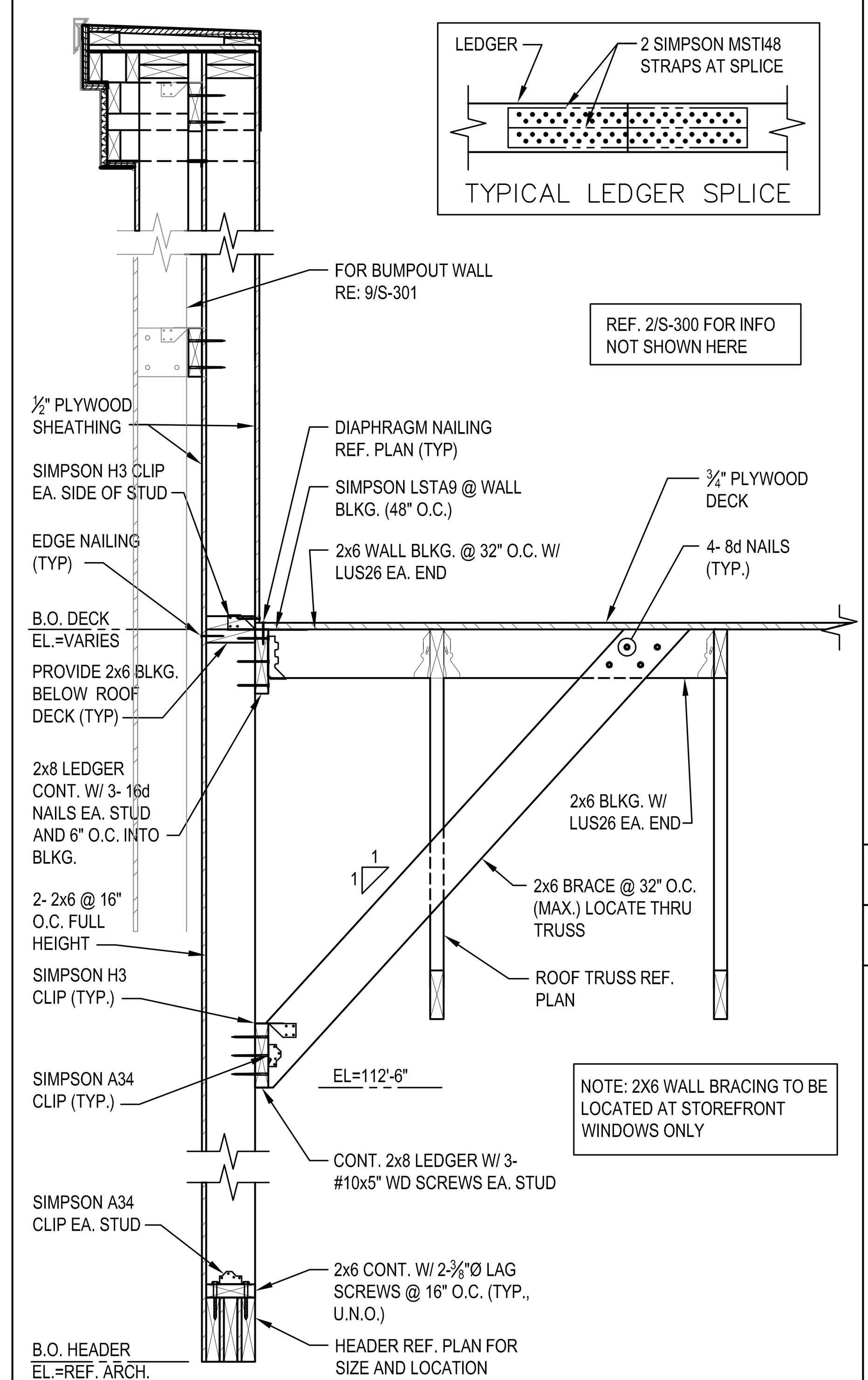
SECTION 9

SCALE: 3/4"=1'-0" S–300



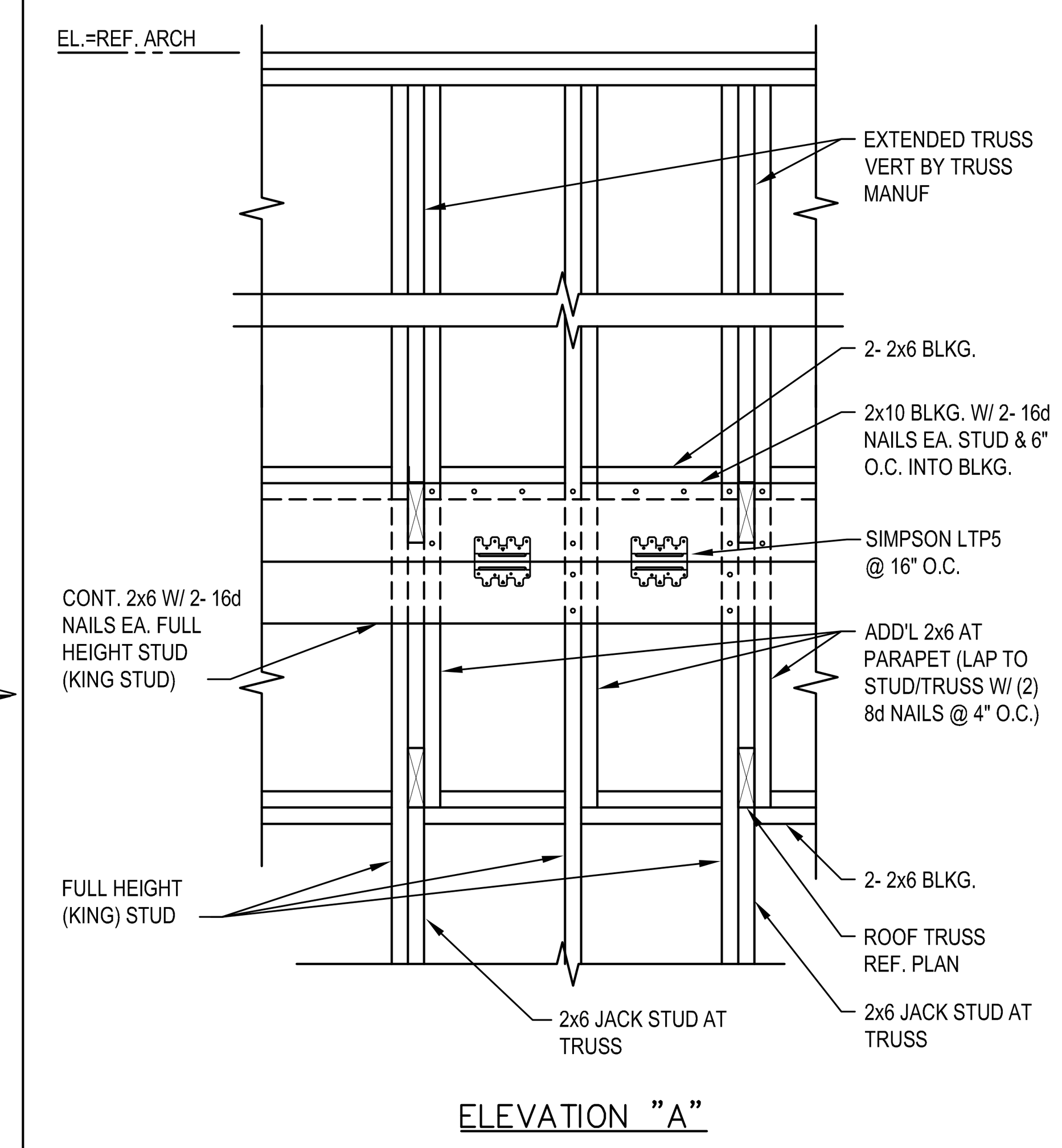
SECTION 5A

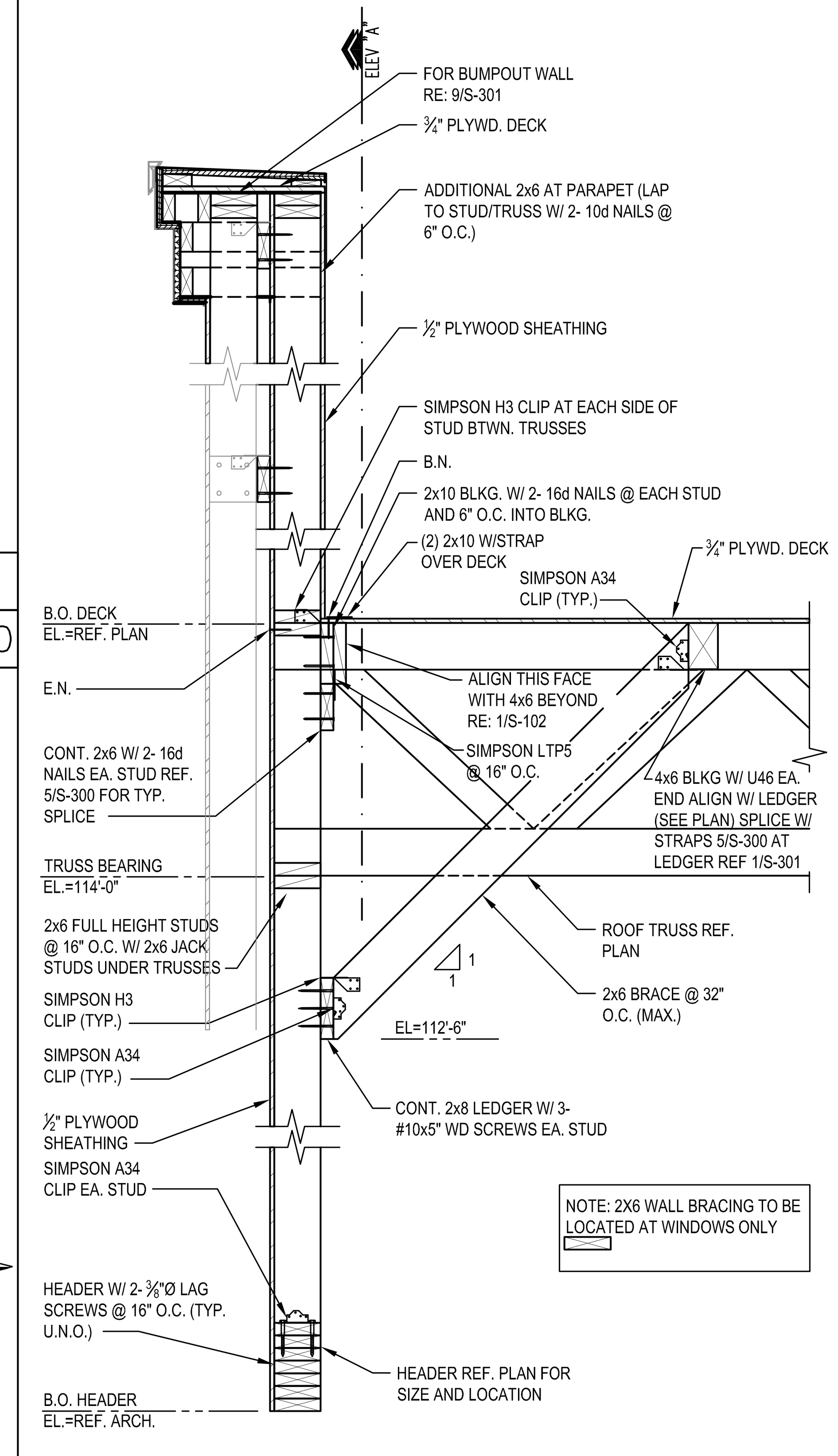
SCALE: 3/4"=1'-0" S–300



SECTION 5

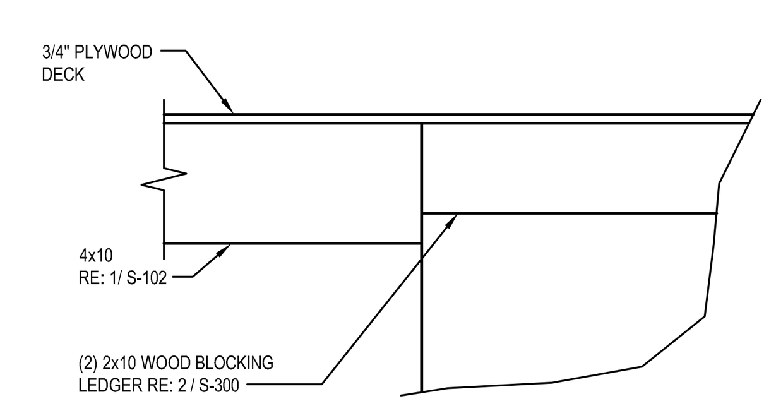
SCALE: 3/4"=1'-0" S–300





SECTION 2

SCALE: 3/4"=1'-0" S–300



SECTION 1

SCALE: 3/4"=1'-0" S–300

CFT DEVELOPMENT, LLC.
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Rosemead, California
91770
Telephone: 626.799.9898
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REVISIONS:

ISSUE DATE:

PERMIT SET 2020-11-13

DRAWN BY: DWG

CFT PROJECT #:

ARCH PROJECT #: JCDT190143

I certify that these documents were prepared or approved by me, and that I am a duly licensed architect or engineer under the laws of the State of Maryland, license number 38483, expiration date 02-17-2022.

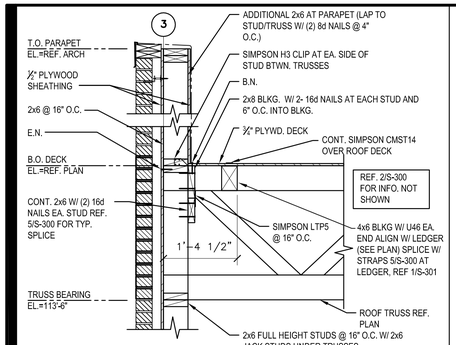
NORR ARCHITECTS ENGINEERS PLANNERS

CFT PLAZA
S-19-0029

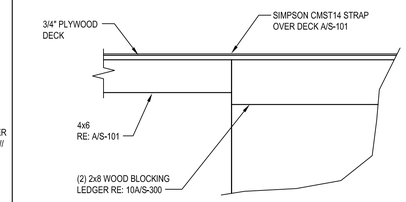
210 MALCOLM DR.
WESTMINSTER, MD 21157

S-300

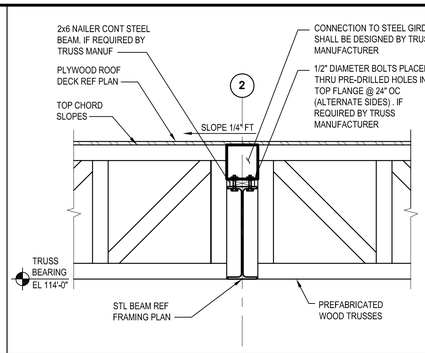
FRAMING SECTIONS



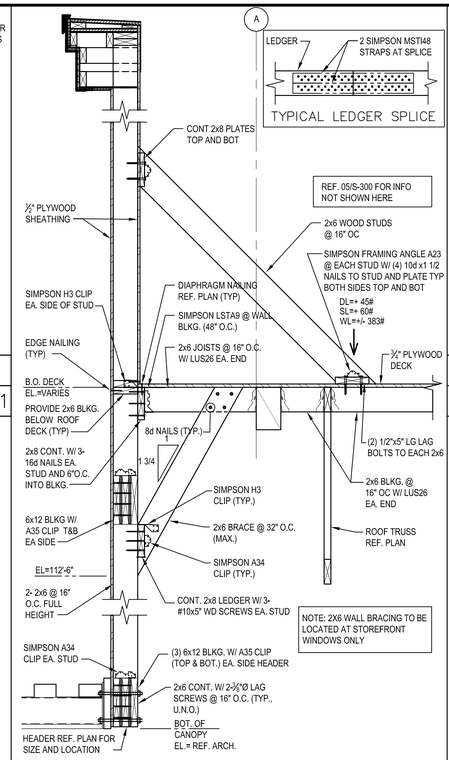
SECTION	16
	S-301



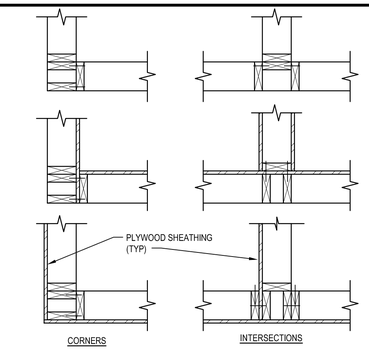
SECTION	15
	S-301



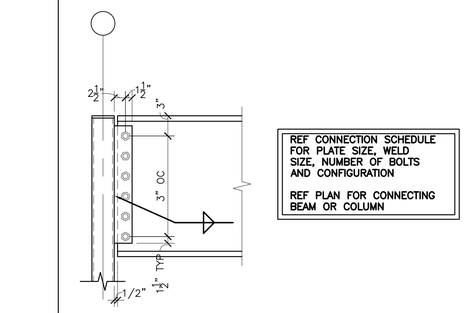
SECTION	12
	S-301



SECTION	7
SCALE: ½"=1'-0"	S-301



TYPICAL WALL FRAMING	4
SCALE: ½"=1'-0"	S-301



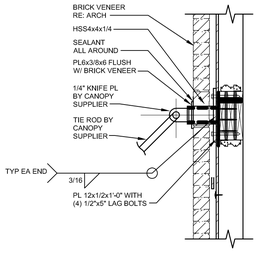
NOTES:

1. ALL BOLTS TO BE 3/4" DIAMETER A325 N, UNLESS OTHERWISE NOTED.
2. ALL WELDS EACH SIDE WITH E70XX ELECTRODES. DO NOT RETURN WELDS.
3. SHORT SLOTTED HORIZ HOLES ARE PERMITTED.

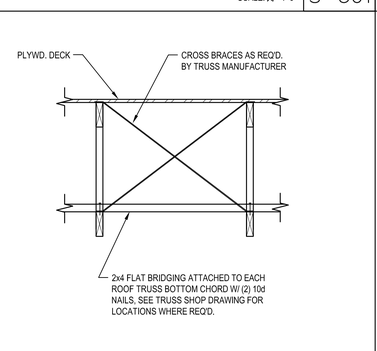
BEAM CONNECTION SCHEDULE

BEAM SIZE	PLATE THICKNESS	WELD SIZE	# BOLTS
W21	1/2"	5/16"	6
W18	3/8"	1/4"	5
W16	3/8"	1/4"	4

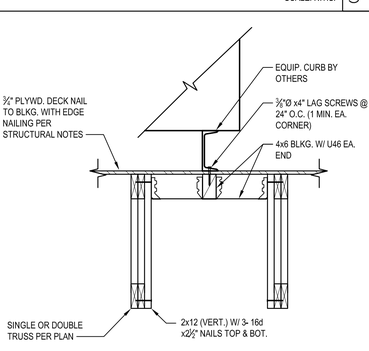
BEAM CONNECTION	14
SCALE: ½"=1'-0"	S-301



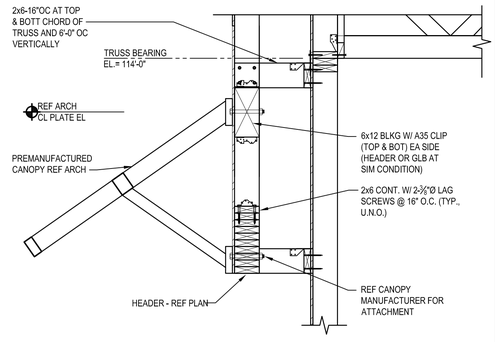
CANOPY ATTACHMENT	11
SCALE: ½"=1'-0"	S-301



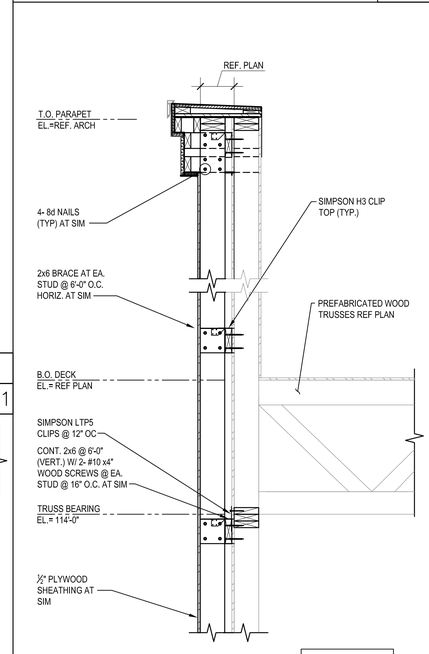
TYPICAL BRIDGING AND BRACING	6
SCALE: ½"=1'-0"	S-301



TYPICAL FRAMING NOTCHES	3
SCALE: N.T.S.	S-301



CANOPY FRAMING	13
SCALE: ½"=1'-0"	S-301



SECTION	9
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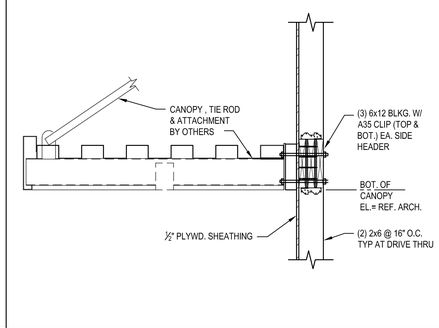

CANOPY BLKG	5
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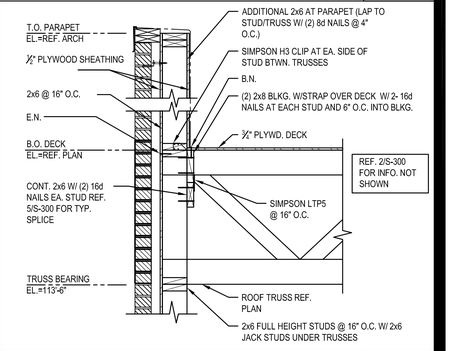

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	S-301

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I certify that these documents were prepared or approved by me, and that I am a duly licensed architect or engineer under the laws of the State of Maryland, license number 38483 expiration date 02-17-2022.

NORR
ARCHITECTS | ENGINEERS | PLANNERS

CFT PLAZA
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210 MALCOLM DR.
WESTMINSTER, MD 21157

S-301

TYPICAL FRAMING DETAILS & SECTIONS